

**BRIEF DESCRIPTION OF FLOOD PROTECTION BARRIERS:**

TYPE: ACTIVE (REQUIRES HUMAN INTERVENTION)

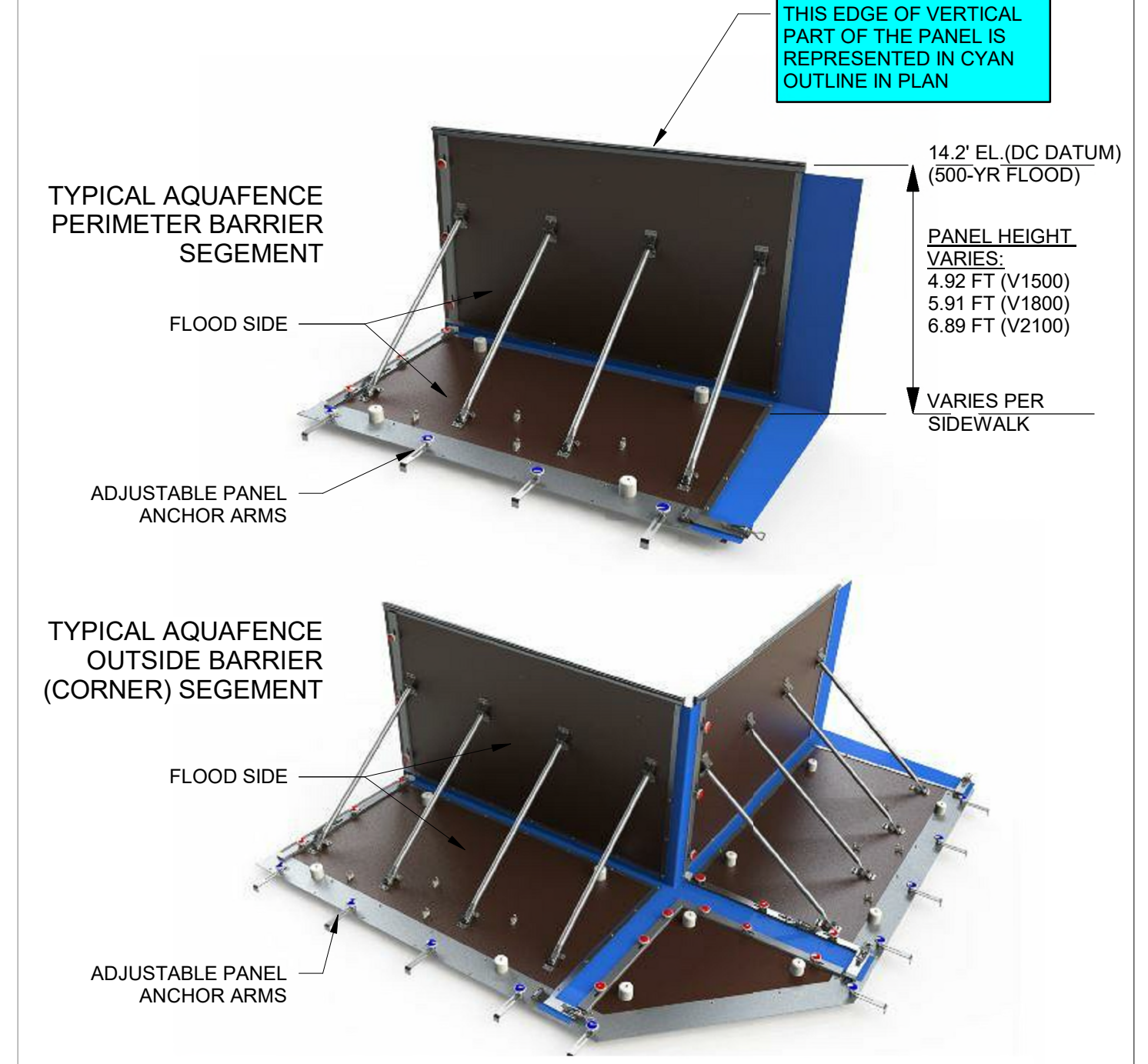
REMOVABLE: YES

FM APPROVED: YES

TOP OF FLOOD BARRIER PANEL: 14.2' DC DATUM (14.1' NAVD 88) MIN.

SIDEWALK ATTACHMENT REQUIRED: YES. INTERNALLY-THREADED TIE-DOWN ANCHORS TO SECURE THE TOE OF AQUAFENCE PANELS TO ASPHALT OR CONCRETE PAVEMENTS, 3/8 OR 1/2" DIAMETER (SEE BELOW)

DESCRIPTION: PANELS DESIGNED FOR RAPID DEPLOYMENT, CONSTRUCTED OF MARINE GRADE LAMINATE, STAINLESS STEEL ALUMINUM AND REINFORCED PVC CANVAS.



**AquaFence** What types of ANCHORS do we use?

AquaFence recommends using internally-threaded tie-down anchors to secure the toe of AquaFence panels to asphalt or concrete pavements. The anchors should be flush with the pavement surface to prevent damage from traffic, and removable caps should be installed to protect the threads and prevent debris from entering the threaded holes. Portions of pavement anchors embedded in the ground should be zinc-plated or stainless steel to resist corrosion. Anchor bolts can be stored with the panels for quick deployment. Anchoring systems should be designed by an engineer and be load-tested to verify capacity. See side 2 of this sheet for a typical anchoring sequence of operations.

**01 Asphalt Pavement Anchor**

In asphalt pavements, specially designed asphalt pavement anchors for 3/8" or 1/2" diameter bolts are inserted into cored holes and secured with fast-setting grout or epoxy. Anchors made by Asphalt Anchors Corp. are examples of asphalt pavement anchors. Anchor sizes will depend on the anchor loads needed and the pavement construction.

**Anchor Specs**

- 2inc Plated or Stainless Steel
- Inside Thread 3/8" or 1/2" Diameter
- Plastic Cap
- 3/8" or 1/2" Diameter to Match Anchor Used
- Anchor Bolt
- 2inc Plated or Stainless Steel
- 3/8" or 1/2" Diameter
- Washer
- 2inc Plated or Stainless Steel

**02 Concrete Pavement Anchor**

In concrete pavements, a number of internally-threaded, drop-in or adhesive-type anchor systems for 3/8" or 1/2" diameter bolts can be used, depending on the required anchor loads and strength of concrete. The same types of anchors can also be used for bolting to concrete walls.

**Anchor Specs**

- Drop-In Anchors
- Stainless Steel
- Non-Flange
- Inside Thread 3/8" or 1/2" Diameter
- Stainless Steel Cap Screw
- Flat Head - Tapered
- 3/8" or 1/2" Diameter to Match Anchor Used
- Anchor Bolt
- 2inc Plated or Stainless Steel
- 3/8" or 1/2" Diameter
- Washer
- 2inc Plated or Stainless Steel

**Attaching to Typical Embedded Anchor**

The following shows a typical installation sequence for attaching AquaFence panels to embedded anchors in asphalt or concrete pavements. Panel anchor arms are adjustable. Panels do not need to be in exact location to line up with anchors.

- Remove cap. Clean area. Prepare anchor bolts and nuts.
- Place AquaFence panels in position. Place attachment arm directly over the embedded anchor so that the open slot aligns with the anchor.
- Install bolt with washer through open slot in attachment arm.

These three steps are the same for embedded anchors in asphalt or concrete pavements.

Asphalt Pavement Concrete Pavement

DOB APPROVAL STAMPS:

No.	Date	Description
10	12/14/18	IFC
5	07/16/18	Final GMP & Coordination Set
4	04/10/18	95% CD

**Submissions & Revisions**

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Phone: 212.285.0088

General Contractor  
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Phone: 301-680-3200

Project Location  
**2100 2nd St, SW**  
Washington, DC

Drawing Title  
**FLOOD PROTECTION PLAN**

Seal  
**DISTRICT OF COLUMBIA**  
Professional Engineer  
No. 5826  
Date: 12/14/18  
Drawn By:  
Checked By:  
Project No:  
Drawing No. **0020**



FLOODPLAIN NARRATIVE

The DC Flood Hazard Rules are presented in Title 20 DCMR, Chapter 31, and outline the requirements for development in a regulatory floodplain with the purpose of promoting the public health and safety...

The Flood Hazard Rules do not specifically allow for proposed mixed-use development within a regulatory floodplain. Therefore, as a mixed-use development proposed with below grade parking, the applicant must apply for a Code Modification in accordance with DCR...

Base Flood Elevation (BFE): The BFE associated with the Potomac River at Buzzard Point is 10.6 feet (NAVD 88).

Design Flood Elevation (DFE): The DFE agreed to between the Applicant and DOEE for this Site is the 500-year flood elevation of 14.1 feet (NAVD 88).

Encroachment Analysis: Due to the Site's location at the confluence of the Anacostia and Potomac Rivers, the flood elevation at this location along the Anacostia River is a backwater effect from the Potomac River...

Lowest Residential Floor Elevation: The lowest floor elevation of residential units is required to be at or above the DFE. The lowest floor shall be at elevation 32.53 feet DC Datum (32.43 feet NAVD 88)...

Utilities and Ventilation Openings: All utilities and ventilation openings are required to be at least 1.5 feet above the BFE and therefore will be installed above elevation 12.1 feet (NAVD 88).

Floodproofing: Floodproofing is required up to at least the DFE. Floodproofing shall consist of utilizing flood resistant construction materials in combination with dry-floodproofing up to an elevation of 14.1 feet (NAVD 88).

FLOOD PROTECTION NOTES

- 1. THE FLOOD EMERGENCY PLAN (FEP) SHALL BE POSTED PERMANENTLY IN AT LEAST TWO (2) CONSPICUOUS LOCATIONS WITHIN THE STRUCTURE.
2. PROVISIONS WILL BE MADE FOR THE EVACUATION OF ALL OCCUPANTS BEFORE THE BUILDING IS ISOLATED BY FLOOD WATERS.
3. PROPOSED REFUGE AREAS DURING A FLOOD EVENT HAVE BEEN DESIGNATED FOR OCCUPANTS WHO DO NOT EVACUATE PRIOR TO THE ONSET OF FLOODING.

REFUGE AREA NO. 1: LOCATED AT V STREET, SW. ESTIMATED CAPACITY IS 70 OCCUPANTS STANDING SPACE MIN. ALUMINUM CROSSOVER PORTABLE LADDER TO BE PROVIDED AT EACH REFUGE AREA TO REACH OVER THE FLOOD BARRIER.

REFUGE AREA NO. 2: LOCATED ON 1ST STREET, SW. ESTIMATED CAPACITY IS 70 OCCUPANTS STANDING SPACE MIN. ALUMINUM CROSSOVER PORTABLE LADDER TO BE PROVIDED AT EACH REFUGE AREA TO REACH OVER THE FLOOD BARRIER.

- 4. THIS PROJECT IS NOT A CRITICAL FACILITY SUCH AS A HOSPITAL OR A POLICE STATION. SHELTERING IN PLACE IS NOT ALLOWED. CLOSEST EVACUATION SHELTER IS JEFFERSON MIDDLE SCHOOL LOCATED AT 801 7TH STREET SW PER HSEMA SHELTER & EVACUATION GUIDE FOR WARD.
5. STORAGE FOR REMOVABLE FLOOD BARRIERS IS ALLOCATED WITHIN P1 & P2 PARKING LEVELS.
6. FOR PRECISE SPOT ELEVATIONS PLEASE REFER TO CIVIL DRAWINGS. SHEET CIV0107. SPOT ELEVATIONS SHOWN ARE IN DC DATUM. FOR CONVERSION FORMULA INTO NAVD 88 DATUM SEE FLOODPLAIN CHART ABOVE.

An Operation and Maintenance (O&M) Program will be established outlining the maintenance requirements and operational procedures necessary for the use of such a system. In addition, a Floodproofing Certificate shall be provided at completion of the project to certify that the entire building perimeter is floodproofed to the DFE of 14.1 feet, as required.

Storage of Materials: Storage for all hazardous materials (required for general maintenance and cleaning of the facility) will be provided within the Janitor's closet located on the first floor and the Mechanical Room/ Water heater room on the Penthouse level, both within the floodproofed building. There will be no other storage of hazardous materials outside of the floodproofed building at this Site.

Means of Egress:

Two separate exits, one at V Street and second exit on 1st Street, have been provided from the building. V Street exit is along the North Face facade of the building and is at elevation of 9.95 feet DC Datum (9.85 feet NAVD 88). 1st Street exit is along the East Façade of the building and is at elevation of 9.95 feet DC Datum (9.85 feet NAVD 88). Road grades along V Street generally range from elevation 8.6' to elevation 9.9' along the property.

Due to the grades of the surrounding streets, flood depths between the building and V Street for the 100-year and 500-year flood events will be approximately 1.5 feet and over 5 feet, respectively. Flow velocities at Buzzard Point are expected to be minimal due to backwater conditions from the Potomac River and therefore flood depths up to approximately 2.5 feet are assumed to be acceptable for emergency personnel and residents to wade through flood waters, if necessary.

There will be no sheltering in place and evacuation of building tenants is mandatory in case of a flood emergency.

Refuge Space: Refuge space shall be provided on the ground level in two locations in the event residents will need to be evacuated during a flooding event. One area is located along V Street at the main entrance with space to accommodate 70 people. A second area is located along 1st Street with space to accommodate 70 people.

Flood Warning System: A roof top rainfall monitor will be installed on the roof. The system will allow for remote monitoring at the 24 hour staffed concierge desk in the lobby. The National Weather Service's email alert service will be utilized as the flood warning system that will notify operation and maintenance personnel, as well as residents of the potential for flooding events.

Operations and Maintenance (O&M) Program: An O&M Plan will be developed and kept on site with operations and maintenance personnel that will outline protocol for monitoring potential flood events, ensuring a continual maintenance schedule, performing scheduled maintenance on flood warning and flood protection systems, and updating the Emergency Action Plan information as needed.

Emergency Action Plan (EAP): An EAP shall be developed to identify stages at which emergency action is to be taken, including notification of key personnel for the installation of manual portions of the flood protection system. The EAP shall outline specific protocol for who is responsible for initiating certain notification procedures and list key contact information for responsible parties.

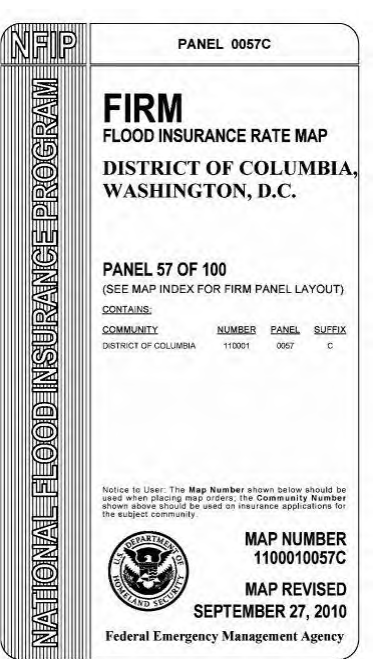
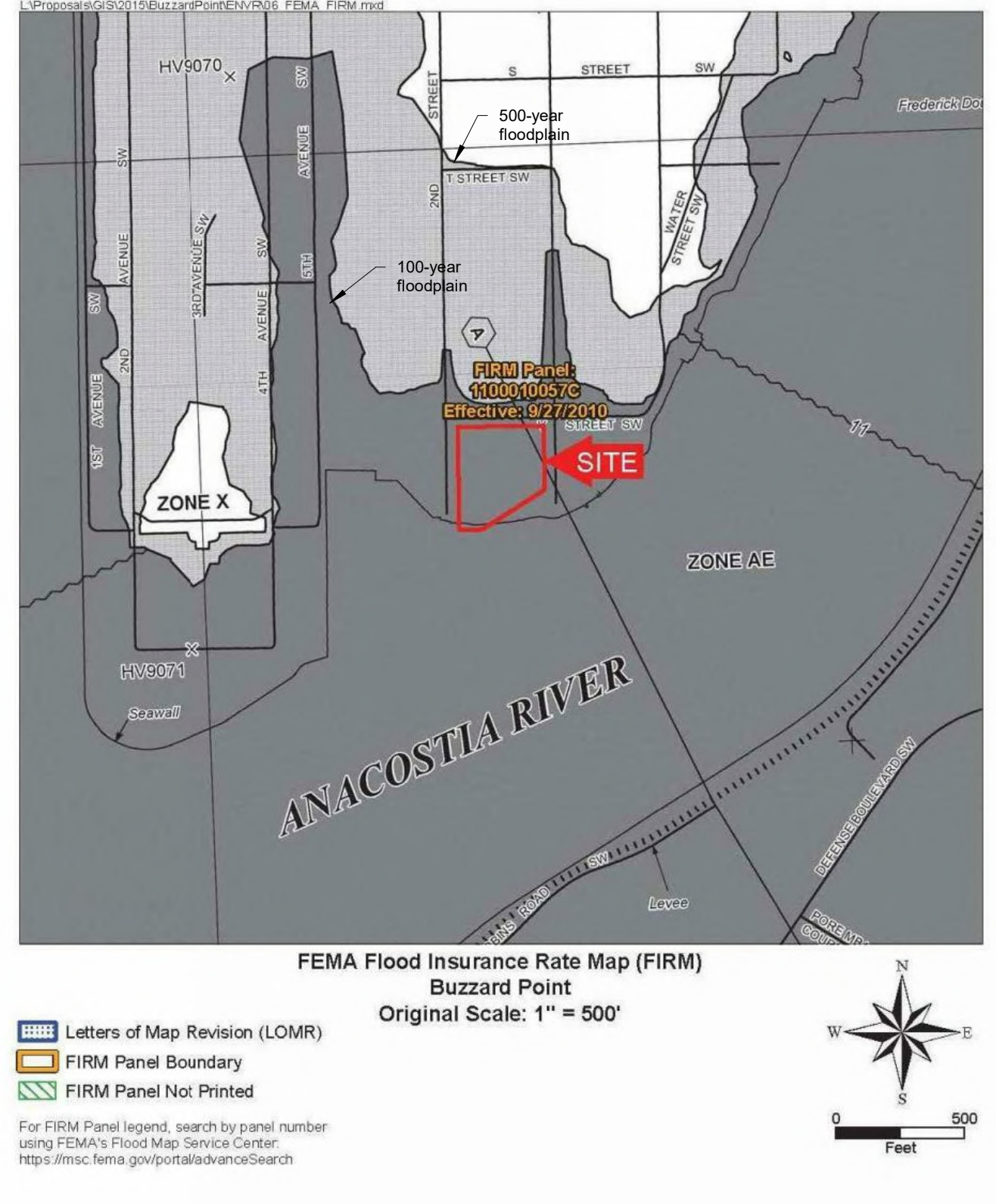
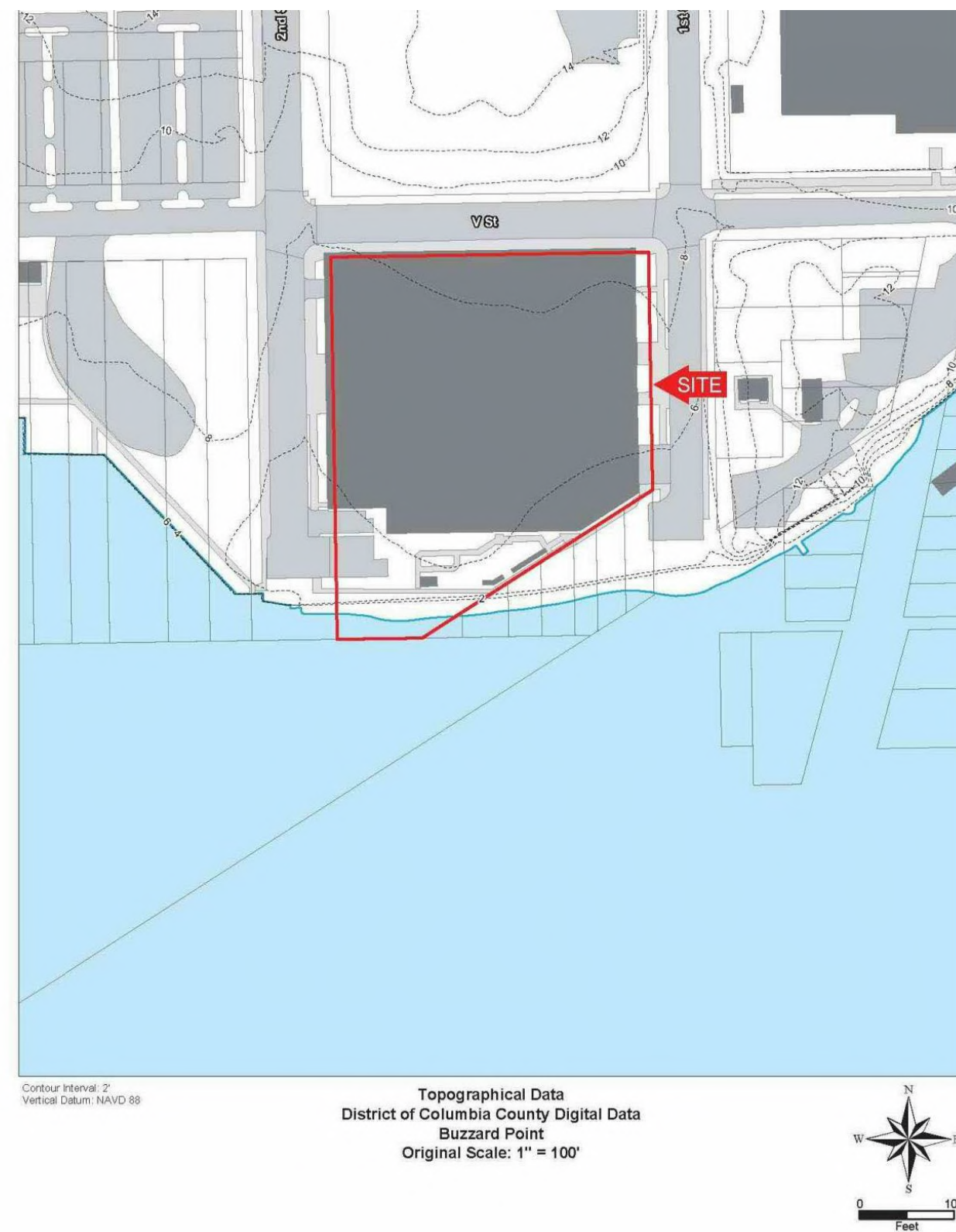
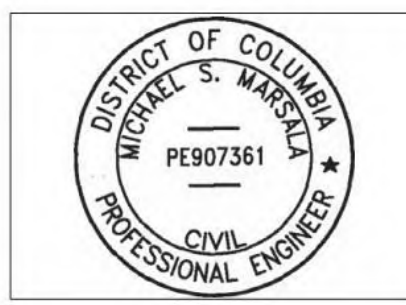
FLOODPLAIN INFO table with columns: FLOOD ZONE, FIRM MAP/PANEL NUMBER, MAP REVISED, 100-YR FLOODPLAIN, 500-YR FLOODPLAIN, CONVERSION FORMULA FROM NAVD88 TO DC DATUM.

BUILDING LEVELS table with columns: LEVEL/FLOOR, DC DATUM, NAVD 88. Rows include P2 - PARKING LEVEL, P1 - PARKING LEVEL, LEVEL 01 - GROUND, LEVEL 02 - CONTAINS MAIN ELECTRICAL ROOM ONLY, LEVEL 03 - LOWEST FLOOR WITH RESIDENTIAL DWELLING UNITS, LEVEL 04, LEVEL 05, LEVEL 06, LEVEL 07, LEVEL 08, LEVEL PH (PENTHOUSE).

STATEMENT BY PROFESSIONAL ENGINEER REGISTERED IN THE DISTRICT OF COLUMBIA

This is to certify that the elevation information data, hydrologic and hydraulic analyses, and any other supporting data of this floodplain management plan and report have been examined and performed by me and found to be in conformity with modern engineering principles. I further certify that all project works are designed in accordance with sound engineering practices to provide protection from the base flood (also referred to as the "one hundred year flood") and are in compliance with other standards and regulations, in accordance with the specifications required under Title 20 DCMR, Chapter 31. I understand that any false statement may be punishable by fine or imprisonment, pursuant to D.C. Official Code § 22-2405.

Michael S. Marala, Registered Professional Engineer, No. 9707361, District of Columbia. Address: 5300 Wellington Branch Drive, Gainesville, VA 20155. Date: 01-14-2018. Phone No: 703-679-5656.



ELEVATION CERTIFICATE form for River Point Partners, LLC. Includes sections for Section A - Property Information, Section B - Flood Insurance Rate Map (FIRM) Information, and Section C - Building Elevation Information (Survey Required).

ELEVATION CERTIFICATE form for River Point Partners, LLC. Includes sections for Section D - Surveyor, Engineer, or Architect Certification, and Section E - Building Elevation Information (Survey Not Required) for Zone AO and Zone A (Without BFE).

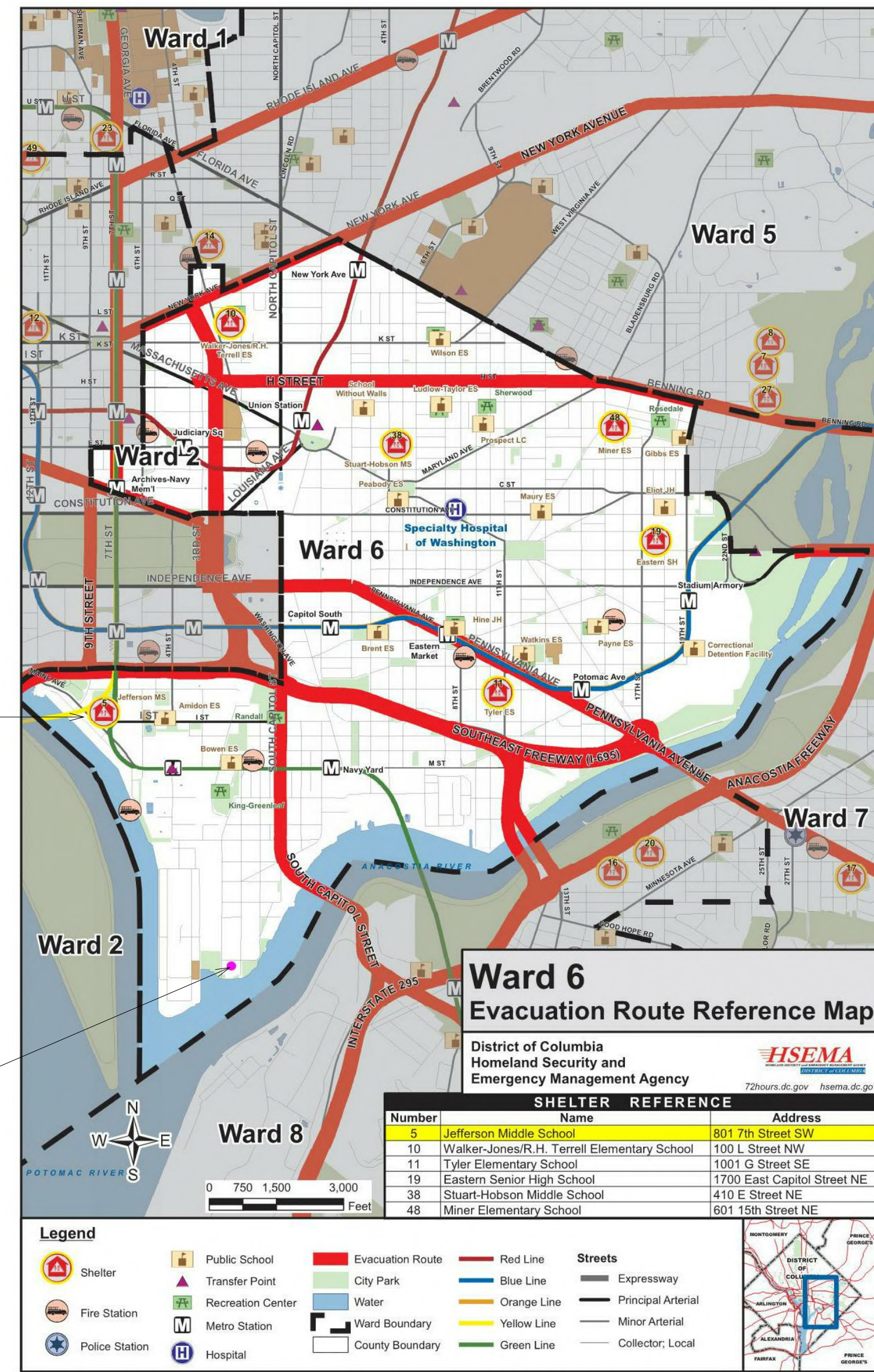
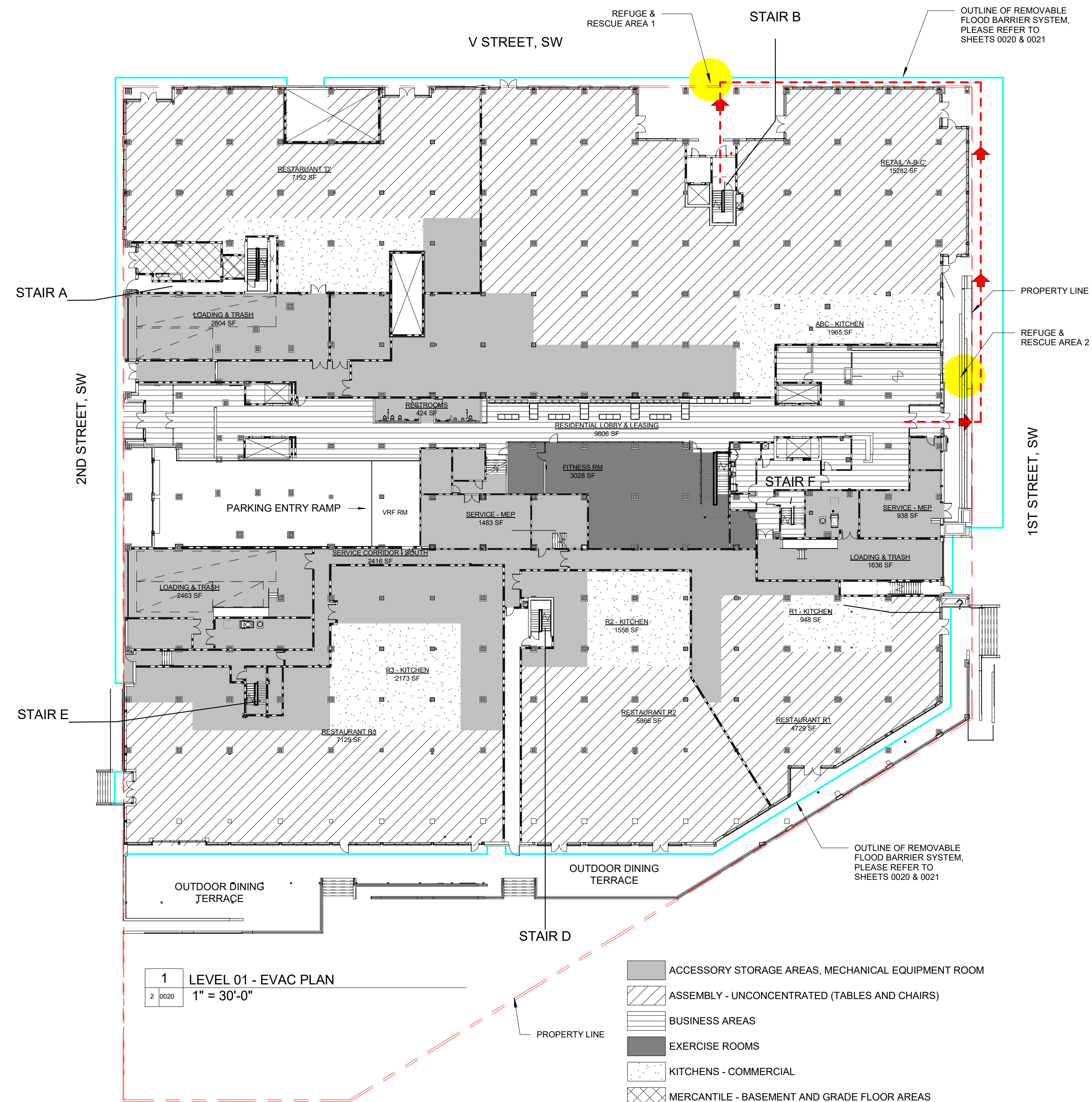
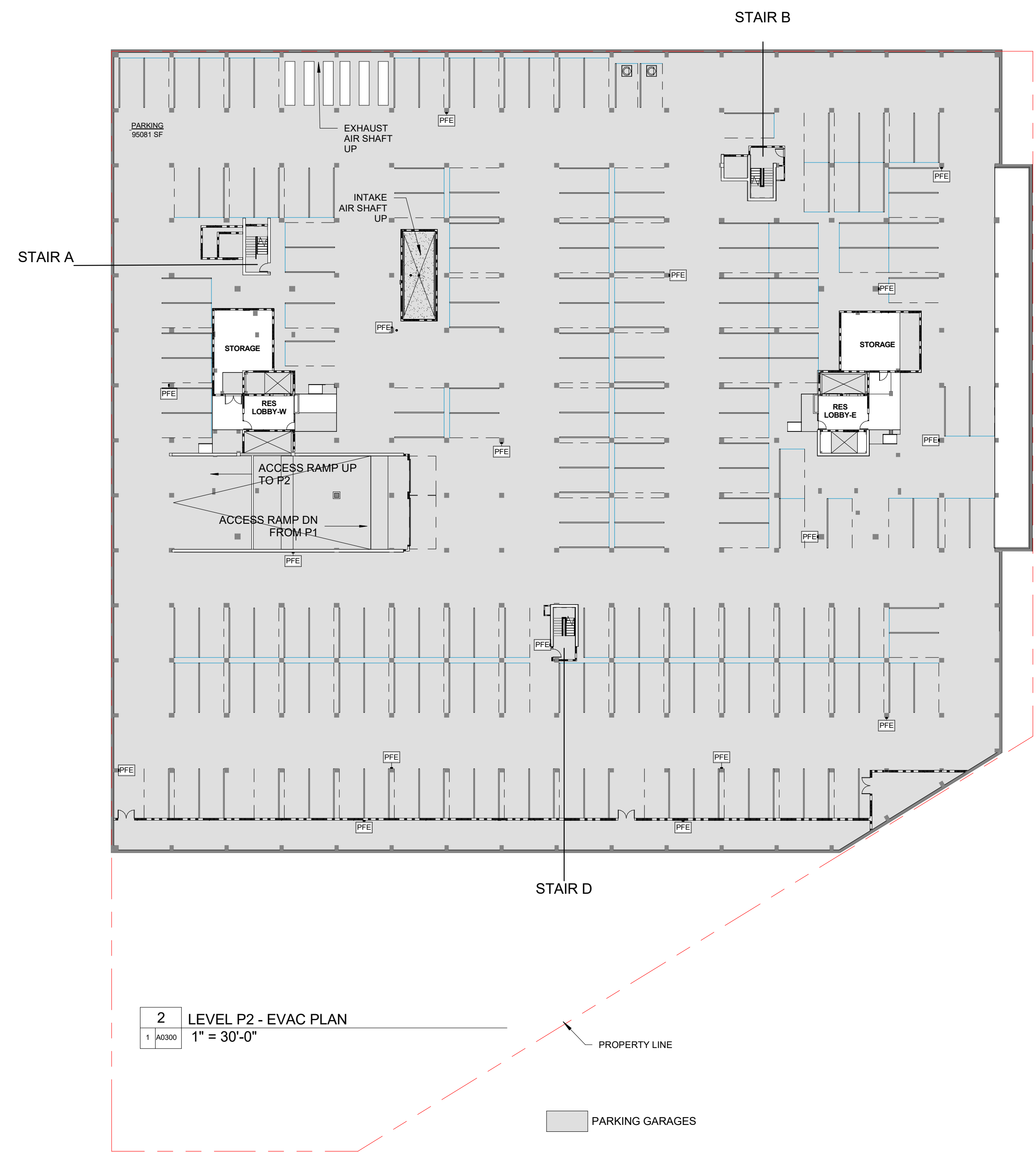
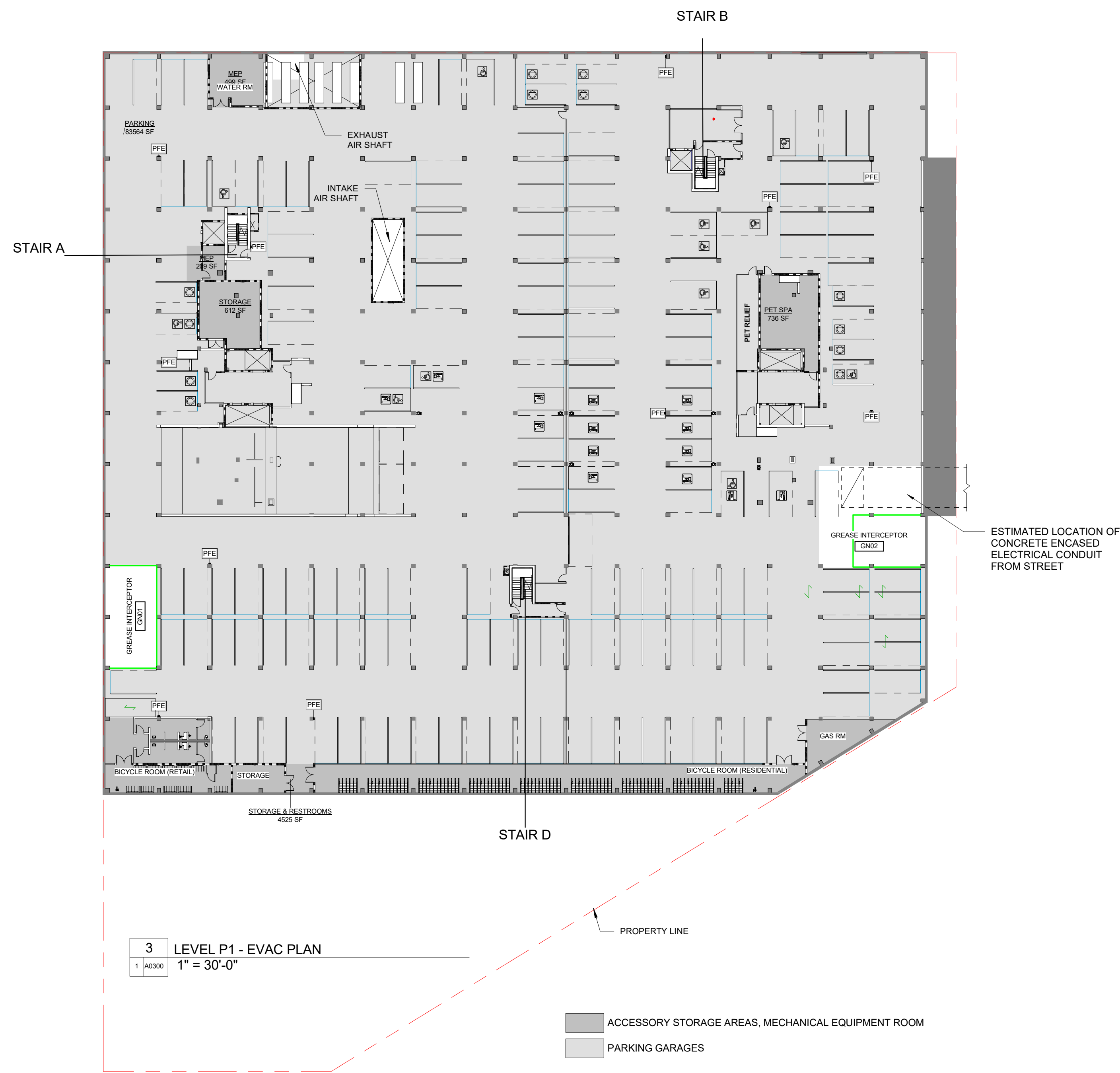
ELEVATION CERTIFICATE form for River Point Partners, LLC. Includes sections for Section F - Property Owner (or Owner's Representative) Certification, and Section G - Community Information (Optional).

ELEVATION CERTIFICATE form for River Point Partners, LLC. Includes sections for Section H - Building Photographs, Section I - Building Photographs, and Section J - Building Photographs.

ELEVATION CERTIFICATE form for River Point Partners, LLC. Includes sections for Section K - Building Photographs, Section L - Building Photographs, and Section M - Building Photographs.

DOB APPROVAL STAMPS: Includes a table of approval dates and descriptions, and a large '0021' stamp from the District of Columbia Department of Buildings.





- GENERAL NOTES:**
- THIS PROJECT IS **NOT A CRITICAL FACILITY** SUCH AS A HOSPITAL OR A POLICE STATION. SHELTERING IN PLACE IS NOT ALLOWED. CLOSEST EVACUATION SHELTER IS JEFFERSON MIDDLE SCHOOL, LOCATED AT 801 7TH STREET SW PER HSEMA SHELTER & EVACUATION GUIDE FOR WARD 6
  - THERE ARE (2) TWO BELOW GRADE PARKING LEVELS - P1 AND P2. EGRESS FROM P1 AND P2 IS VIA (3) EGRESS STAIRS:
    - STAIR A - EXITS WITHIN FLOOD PROTECTED ENCLOSURE ON 2ND STREET & LEADS TO REFUGE AREA 1 ON V STREET
    - STAIR B - EXITS WITHIN FLOOD PROTECTED ENCLOSURE ON V STREET & AT REFUGE AREA 1
    - STAIR D - EXITS THROUGH SERVICE RETAIL CORRIDOR DURING A FLOOD EMERGENCY & CONNECTS TO FLOOD PROTECTED ENCLOSURE ON 2ND STREET WHICH LEADS TO REFUGE AREA 1 ON V STREET
  - FOR RESIDENTIAL AND LODGING OCCUPANTS ABOVE GROUND FLOOR EGRESS IS PROVIDED VIA (5) EGRESS STAIRS:
    - STAIR A,B,D - SAME AS ABOVE
    - STAIR C - EXITS WITHIN FLOOD PROTECTED ENCLOSURE ON 1ST STREET AND LEADS TO REFUGE AREA 1 & 2
    - STAIR E - EXITS WITHIN FLOOD PROTECTED ENCLOSURE ON 2ND STREET & LEADS TO REFUGE AREA 1 ON V STREET
  - EVACUATION WARNING SYSTEM WILL BE TIED INTO FLOOD FORECASTING SYSTEM.

DOB APPROVAL STAMPS:

10	12/14/18	IFC
5	07/16/18	Final GMP & Coordination Set
No.	Date	Description

Submissions & Revisions

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Project Location  
**2121 1st St, SW**  
Washington, DC

Drawing Title  
**FLOOD PROTECTION EVACUATION PLAN**

Seal  
**DISTRICT OF COLUMBIA**  
Professional Engineer  
No. 5826  
ANTUNOVICH ASSOCIATES  
ARCHITECT

Date: 02/14/2020  
Drawn By:  
Checked By:  
Project No:  
Drawing No. **0022**









August 27, 2018

Semere Hadera, PE  
Structural Eng. Supervisor, POD  
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1100 4th St, SW  
Washington, DC 20004  
ph. 202-442-4673  
semere.hadera@dc.gov

Re: Project Address: 2100 2nd St, SW Code Modification MVQ1800081  
Responses to structural comments dated August 22, 2018

Dear Mr. Hadera,

Below please find responses related to the design of structural elements associated with Code Modification MVQ1800081 for River Point project located at 2100 2nd Street SW.

1. Introduction:

This document has been prepared in response to structural review comments associated with Code Modification MVQ 1800081 for 2100 2nd Street SW project (Building Permit Application B1803132).

2. Site Description:

The project is located within the 100-year floodplain in flood zone AE, but it will be designed to protect the building up to an increased 500-year floodplain per City's recommendation based on ongoing climate change studies. Flood protection is planned to be achieved via a dry flood proofing method. It will be accomplished through a combination of flood-damage resistant materials and use of removable flood barriers (per flood protection plan previously submitted on sheet 0020) which will be deployed manually in case of a flood event warning.

- 100-year flood elevation = 10.7' el. (DC Datum) OR 10.6' el. (NAVD 88)
- 100-year flood elevation + 1.5 ft of freeboard = 10.7' + 1.5' = 12.2' el. (DC Datum) OR 12.1' el. (NAVD 88)
- 500-year flood elevation = 14.2' el. (DC Datum) OR 14.1' el. (NAVD 88)

Zivan Cohen P.E. | Vladimir M. Abou P.E. | Michael Tabasi P.E. | J. Kelley White | Ali R. Tabbae P.E. | Sanjay Khanna P.E. | Yehuda Neulman P.E. | S.E. | Dushik M. Shah P.E. | Xinyang Li P.E. | Hensha Wijesena | Pinar White P.E. | Jacob Sushchak P.E. | Amir Mahmoud P.E. | Lindsey Ahmed P.E. | Rose V.P.E. | Pinar Sackel | David Troncoso P.E. | Sebastian Jank P.E. | Ervin Rogers, Jr. P.E. | Mustafa Ali Zaki | 1100 Spring Street - 1100 River - Silver Spring - Maryland - 20910-4000

Semere Hadera, PE  
Structural Eng. Supervisor, POD  
Department of Consumer and Regulatory Affairs  
Project Address: 2100 2nd St, SW Code Modification MVQ1800081  
Responses to structural comments dated August 22, 2018

The existing 30 inch thick mat foundation needed to have tie-down (also referred to as micropiles) anchors to withstand uplift forces associated with 500-year flood under the courtyard.

The existing foundation walls were checked, and are found to be able to resist hydrostatic, hydrodynamic and impact forces associated with a 500-year flood. Additionally, there are limited portions of exterior walls at the Southeast and Southwest corners of the building envelope which will be built anew, and will have an 8 to 10 inch thick concrete backup wall in height up to 500-year flood elevation. These new walls are designed to withstand required forces associated with 500-year flood as well.

A majority of the exterior facade has openings consisting of storefront windows and doors. To prevent flood waters from entering the building, removable flood barrier panels are proposed to be installed up to the required 500-year flood elevation. Design of these removable flood barrier panels is by other calculations associated with that design have been also submitted the previous submissions.

B. The existing mat foundation at the transfer girder supporting column for revised increased loads.

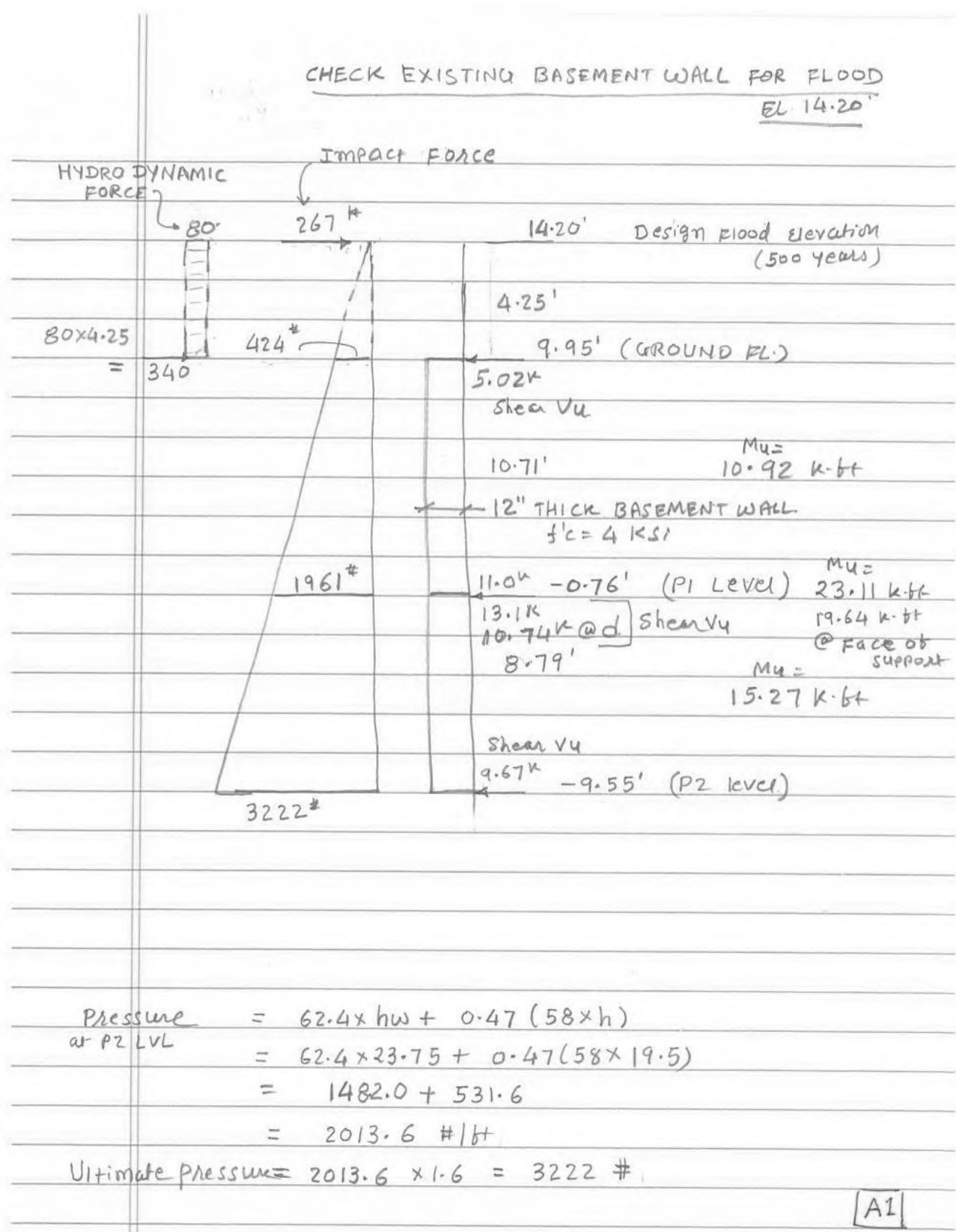
B. The existing columns which support the transfer girder for revised increased loads.

6. Method of Design and Analysis:

To demonstrate that foundation will be able to withstand uplift water pressure Geotechnical Study described in Memorandum No. 3 prepared by ECS was used. It addresses geotechnical design requirements resulting from the 500-year flood. To address the increase in water pressure, ECS evaluated two issues:  
1) the positive effects of soil friction along the basement walls, and  
2) the time rate effects of the flood, and whether or not the flood stage persists for sufficient time to increase water pressure below the mat.

To demonstrate that existing foundation walls and portions of exterior walls that were not protected by removable flood barriers will be able to withstand all necessary forces associated with 500-year flood (see calculations pages A1 to A4).

Hydrostatic and Hydrodynamic forces were calculated to withstand up to 500-year flood elevation. For impact loads due to debris acting on exterior walls that are not protected by removable flood barriers 1,000 lbs force was used in accordance with ASCE 7-10 Eq C5-3, with Velocity of Water not to exceed 5 f/s in accordance with dry flood proofing requirements of ASCE 24-14 Section 6.2.1.



General Beam Analysis table with columns for Description, General Beam Properties, Applied Loads, Design Summary, Maximum Forces & Stresses for Load Combinations, and Vertical Reactions.

Semere Hadera, PE  
Structural Eng. Supervisor, POD  
Department of Consumer and Regulatory Affairs  
Project Address: 2100 2nd St, SW Code Modification MVQ1800081  
Responses to structural comments dated August 22, 2018

Existing Top of P2 Level (Foundation) = -9.55' el. (DC Datum)  
Existing Top of P1 Level = -0.76' el. (DC Datum)  
Existing Top of Ground Floor Level = 9.95' el. (DC Datum)

3. Brief Description and Assessment of the Existing Structure and Structural Elements:

The project's design, orientation and massing are based on the adaptive reuse of the existing building which requires carving out four residential courtyards starting with 3rd floor level to maximize articulation and views for residents and visitors.

- The existing structure is a conventionally reinforced concrete structure consists of:  
a. (2) two below grade parking levels P1 and P2  
b. (8) eight above grade levels

Proposed structural scope of alteration includes:

- a. Existing Foundation: Tie-down anchors (micro piles) will be added at the footprint of four courtyards areas through the existing mat foundation to resist water uplift pressure due to potential flood. A 3-inch concrete topping will be added on top of mat foundation to strength it to resist potential uplift forces and repair existing damage due to wear-and-tear.  
b. Existing 2nd Floor Slab: Demolition of existing 2nd floor slab in its entirety to create a double height space at ground level for Retail Users. Supporting existing columns in these high spaces will be strengthened by enlarging them on all sides with several inches of concrete and rebar (see calculations pages 1 to 10).  
c. Existing 3rd Floor Slab: Partial demolition of existing the 3rd floor slab for the purposes of creating four residential courtyards. Courtyard portions of the slab will be removed and re-poured to strength it to resist landscaping and paving within the courtyard.  
d. Existing 4th through 8th Floor Slabs: (4) four courtyards will be carved out of the existing floor slabs to provide light to the residential units.

Semere Hadera, PE  
Structural Eng. Supervisor, POD  
Department of Consumer and Regulatory Affairs  
Project Address: 2100 2nd St, SW Code Modification MVQ1800081  
Responses to structural comments dated August 22, 2018

7. Proposed Solution:

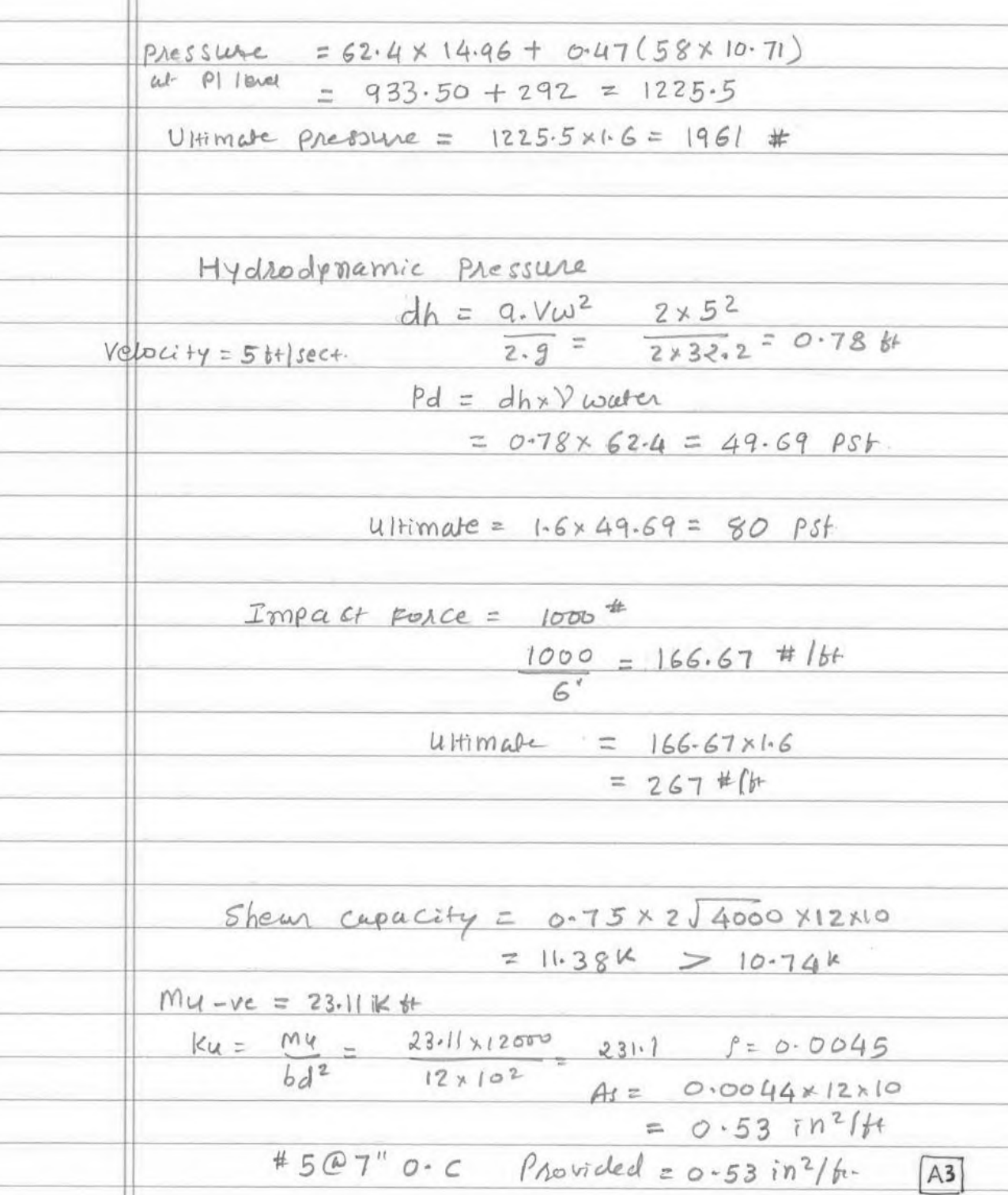
- A. For the existing structure to be able to withstand forces (uplift, hydrostatic, hydrodynamic and impact) associated with 500-year flood event the following alterations are being proposed:  
a. Addition of tie-down anchors through the existing mat foundation at four courtyards area (structural sheet 3001, B1810745 and see structural calculations pages 100, 101, 102 to 108).  
b. Addition of 3" concrete topping on top of the existing mat foundation (P2 level) and add 6" concrete topping slab on top of the new 14" penthouse level slab at tower area (see calculations pages 96, 97 and 101 to 108).  
c. Addition of 8 to 10 inch thick concrete backup walls up to 500-year flood elevation of 14.2' in limited select locations where building facade is not protected by removable flood barriers. These locations are the exterior perimeter at Southeast and Southwest corners of the building envelope.  
B. The existing mat foundation at the transfer girder supporting column needed to be strengthened for revised increased loads. Increase the mat thickness by 3" structural topping slab with rebar and strengthen the existing column with enlarging on all four sides by 5" of concrete and rebar (see calculation pages 87 to 95).  
C. Check the existing column which support the transfer girder needed to be strengthened for increased loads. Strengthen the existing column with enlarging on all sides by 5" of concrete and rebar (see calculation pages 17 to 66).

8. List of Referenced Codes & Standards:

ASCE 7-10 - Minimum Design Loads for Buildings and Other Structures  
ASCE 24-14 - Flood Resistant Design for Construction  
2013 District of Columbia Building Code, Section 1612 - Flood Loads

9. Requirements of ASCE-7 10, ASCE-24 and DCBC 1612:

Please refer to structural calculations that demonstrate compliance with referenced standards.



Semere Hadera, PE  
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- e. Existing Roof Slab (New Penthouse floor level): Demolition of existing roof slab in its entirety and re-pouring of a new 14" thick concrete roof slab with 6" of concrete topping slab (total thickness of concrete = 20"). The topping slab is added to resist the flood uplift forces. Addition of a Penthouse Level on top of re-poured slab.

- f. New Penthouse roof: The Penthouse roof is composite steel construction (3.25" concrete over 3" steel deck supported on steel beams and columns.

- g. New Shear Walls: New shear walls will be added from top of mat to underside of third floor slab to resist the lateral forces.

- h. New Stair and Elevator Slab Openings: New stair and elevator slab openings above 3rd floor will be supported by new steel beams and steel columns as required.

- i. Slab extension on 3rd through Penthouse Floor (ridgelines 14 to 15): One bay will be added at south side of the building, starting at level 3 and continue up to new Penthouse level. The added bay will be an 8-inch thick concrete slab with concrete beams ranging in depth from 20 to 24 inches. Extended south bay will be supported by new concrete columns on the southern edge. The existing columns were checked for the additional loads (see calculations pages 67 to 86).

4. Problem statement:

The removal of portions of existing concrete slabs associated with the creation of four residential courtyards reduce the building weight resulting the flood uplift forces and resulted in a net uplift is created at the courtyards area.

DOEF requested to have structural elements designed to withstand forces associated with 500-year flood. Those structural elements included:  
a. existing mat foundation,  
b. existing foundation walls, and  
c. limited portions of above grade exterior walls that were not protected by removable flood barriers. Those portions were located at southeast and southwest corners of the building.

5. Challenges:

- A. To prevent uplift existing structure will require to be modified.

Semere Hadera, PE  
Structural Eng. Supervisor, POD  
Department of Consumer and Regulatory Affairs  
Project Address: 2100 2nd St, SW Code Modification MVQ1800081  
Responses to structural comments dated August 22, 2018

10. Conclusion:

Existing structural components will be able to withstand required forces associated with a 500-year flood event provided proposed structural alterations of adding tie-downs and thickened concrete backup walls in limited locations are provided.

Should you have any questions, please don't hesitate to contact our office.

Sincerely,  
Sanjay Khanna, PE  
Principal

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DOB APPROVAL STAMPS: 10/12/14/18 IFC No. Date Description Submissions & Revisions Owner: River Point Partners, LLC Architect: ANTUNOVICH ASSOCIATES Structural Engineer: Tadjar-Cohen-Edelson Associates, Inc. M.E.P. & F.P. Engineers: Metropolitan Engineering Civil Engineer: Wries-Mensch Coporation - DC Landscape Architect: West 8 CBG Building Company Project Location: 2100 2nd St, SW Washington, DC Drawing Title: FLOOD PROTECTION STRUCTURAL NARRATIVE Seal: District of Columbia Department of Consumer and Regulatory Affairs No. 5826 Checked By: Project No. Drawing No. 0024