

# Advisory Neighborhood Commission 3D Government of the District of Columbia 

April 7, 2021

Board of Zoning Adjustment<br>District of Columbia Office of Zoning<br>$4414^{\text {th }}$ Street N.W.<br>Washington, DC 20001

## Re: Case Number: 20380 <br> Property Address: 4457 MacArthur Boulevard N.W. <br> Applicant: Polygon Holdings LLC

To the Members of the Board of Zoning Adjustment:
We write to request that you reject the pending application for a special exception submitted by Polygon Holdings LLC ("Polygon"). As detailed further below, Advisory Neighborhood Commission 3D ("ANC 3D") believes the proposed project will have a substantially adverse effect on the use and enjoyment of neighboring properties, most especially 4459 MacArthur Boulevard N.W.

ANC 3D is also deeply troubled by the dismissive and combative approach that Polygon has adopted, both towards ANC 3D and the community, throughout this process. Polygon has largely refused to respond to legitimate concerns raised by commissioners and community members about the potential negative impacts that Polygon's proposed project might have on the adjacent homeowner and other community members, including the possibility that the project might threaten the physical structure of the neighboring property. Polygon has also neither attempted nor succeeded in articulating compelling reasons why the community should support this project in spite of any negative impacts on neighboring property owners.

At its regularly scheduled and publicly noticed monthly virtual meeting on April 7, 2021, with a quorum present, Advisory Neighborhood Commission 3D voted to send this letter to the Board of Zoning Adjustment requesting that it reject Polygon's application for a special exception with regard to 4457 MacArthur Boulevard N.W. (Case No. 20380).

ANC3D devoted significant time to discussing this project at its January 6, 2021 meeting, including extensive time for questions and comments from commissioners, Polygon's representatives, and interested community members. ${ }^{1}$ The project was

[^0]further discussed at ANC 3D's February 3, 2021 meeting, although this discussion did not focus on the merits of the proposal. ${ }^{2}$ Before and after these meetings, ANC 3D also solicited and received written submissions and comments from Polygon, the neighboring property owner, and members of the community.

## Analysis

Polygon proposes to replace a single-family home that makes up one-half of a duplex-style building with an 8-unit multifamily building. Polygon's property, 4457 MacArthur Boulevard, is located in an RA-1 zone. The property, together with the adjoining home, 4459 MacArthur Boulevard N.W., is shown in the photograph below:


Dr. Dawn Lea, the owner of the adjoining property has raised significant and compelling concerns about the impact of Polygon's plans on the structural integrity of her home, which shares an undivided front porch, common roof, common attic and common basement with Polygon's 4457 MacArthur Boulevard. Other community members, as well as Dr. Lea, also noted the potential, if not properly mitigated, for increased noise, traffic, and other nuisances.

In our judgment, Polygon has not addressed these concerns in a good faith manner, despite being given multiple opportunities to do so orally and in writing. In fact, at times Polygon's representatives were overtly dismissive and disdainful when pressed about concerns raised by Dr. Lea or another community member. In other instances, Polygon simply refused to provide an answer, repeatedly chastising commissioners and community members for asking about topics that Polygon's representatives viewed as beyond the scope of the ANC's review. ${ }^{3}$ Most notably,

[^1]Polygon has declined to address a structural analysis report submitted by Dr. Lea in advance of ANC 3D's April $7^{\text {th }}$ meeting. Prepared at Dr. Lea's request by Yahya Aliabadi, PhD, P.E., a Licensed Professional Engineer in the District of Columbia affiliated with A \& A Structures LLC, the report concluded that the "roof structure [on the 4459 "side" of the duplex] will not be able to carry the future increased snow load and must be reinforced and strengthened if any additional load will be added." (Ex. A.)

In considering the applicable zoning regulations, ${ }^{4}$ ANC 3D is persuaded that the proposed special exception will have a substantially adverse effect on the use and enjoyment of the neighboring property, 4459 MacArthur Boulevard, both during and after construction of the proposed project.

First, ANC 3D is concerned about the construction-related impacts on the adjoining property. As noted, Dr. Lea submitted a structural analysis report prepared by Dr. Aliabadi of A \& A Structures LLC that concluded that her portion of the shared roof cannot handle the increased snow load without, at a minimum, reinforcement and strengthening. In addition, Stephen Dupont, an architect retained by Dr. Lea, testified at ANC 3D's January 6, 2021 meeting that construction of the proposed development would have significant negative effects on the physical structure of Dr. Lea's home, absent significant modifications, and would necessarily impact her use and enjoyment of the property. In our judgment, Polygon did not sufficiently address or rebut these concerns, nor did Polygon adequately explain how the issues identified by Mr. Dupon, Dr. Aliabdi, and others would be mitigated by Polygon.

Second, given that the properties are separated by mere inches and share an undivided front porch, common roof, common attic and common basement, ANC 3D is persuaded that-absent mitigation-there is a much greater likelihood that the redevelopment of 4457 MacArthur as a multifamily property will result in objectionable impacts for the neighboring property owner, particularly with respect to noise, than if Polygon was seeking to redevelop a standalone single-family home. Polygon did not seriously dispute the increased likelihood of objectionable impacts on the owner of 4459 MacArthur due to nature of the duplex-style building. We also note that Polygon did not adequately explain how these impacts, which we are persuaded are likely to occur, would be mitigated.

We are not in a position to evaluate the technical claims made by Stephen Dupont, an architect retained by Dr. Lea, or Dr. Aliabadi, author of the structural analysis report discussed above. However, we take their conclusions very seriously. If correct, it is indisputable that the proposed project will have a substantially adverse

[^2]effect on the neighboring property owner. In our opinion, Polygon's silence—despite repeated requests to address these issues-speaks volumes.

In addition, we note that Polygon has also not made any meaningful attempts to articulate compelling reasons why the community should welcome this project despite any negative impacts it may have. We can only conclude that Polygon made a tactical decision to write off ANC 3D and to simply "go through the motions" with the ANC before proceeding to the BZA. Engaging with community members and the relevant ANC when seeking a special exception should not simply be a hoop that a developer must go through or a box they must check before heading before the "real" decision makers. Regardless of whether an ANC ultimately endorses a project, developers should be expected to meaningfully engage with ANCs before seeking an exception at the BZA. We request that the Board consider Polygon's failure to do so as you consider the pending application.

## Findings

Based on our review of the proposed project, ANC 3D finds that, if granted, the special exception requested by Polygon will have "a substantially adverse effect on the use or enjoyment of . . . [the] abutting or adjacent dwelling or property," i.e., the owner of 4459 MacArthur Boulevard NW, the other half of the duplex. Specifically, ANC 3D finds that:
(a) "the light and air available" to the owner of 4459 MacArthur will "be unduly compromised;"
(b) "the privacy of use and enjoyment" for the owner of 4459 MacArthur will "be unduly compromised;" and
(c) "the proposed addition or accessory structure, together with the original building, or the new building, as viewed from the street, alley, and other public way, shall" "substantially visually intrude upon the character, scale, and pattern" of the adjoining home, 4459 MacArthur Boulevard.

While satisfying all quorum requirements, ANC 3D voted to approve this letter and to designate Commissioner J.P. Szymkowicz to testify before any hearing on this issue.


Paige Ela, Chair

# EXHIBIT A 

## A \& A Structures LLC

22 Holly Leaf Ct. Bethesda MD 20817
Tell: 240-678-5399
aastructure@gmail.com

March 28, 2021

## Ms. Dawn Lea

4459 MacArthur Blvd NW, Washington, DC 20007

Re: Existing roof structure check

Dear Ms. Lea,
Further to your request, I visited your property located at 4459 MacArthur Blvd NW, Washington, DC 20007. I checked the existing structure for applicable loads. Per my observation, the existing roof of the building is constructed with $2 \mathrm{x} 6 @ 24$ " O.C. joists. The joists span approximately 14 ft between the supports.

Per my attached calculations, the joists can barely carry the existing applicable snow load. The future development on the neighboring property will certainly add more snowdrift load to the roof which will be beyond the roof structural capacity.
Therefore, the roof structure will not be able to carry the future increased snow load and must be reinforced and strengthened if any additional load will be added.

Thank you for providing us with the opportunity to be of service.
Sincerely,


Yahya Aliabadi, PhD, PE
A \& A Structures LLC

# Structural Calculations 

for

Roof joist check for snow load

## 4459 MacArthur Blvd NW Washington, DC 20007



Prepared By:
Y. Aliabadi, PhD, PE

E-Mail Address: aastructure@gmail.com

## 1 - Objective or Purpose

The following calculation is for Snow Load calculation and Roof Joist check on property located at 4459 MacArthur Blvd NW, Washington, DC 20007

## 2 - References

2-1 - ASCE 7-10 Minimum design Load for Buildings and other structures.
2-2 - IBC 2013, International Building Code
2-3 - NDS 2009, National Design Specification for Wood construction
2-4 - Field measurments

## 3 - Calculation basis

Sructural calculation is performed per Reference 2-1 to 2-4.

## 4 - Assumptions

There is no unverified assumtion in this calculation. Any assumption made in the body of this calculation will be justified in the same section where assumption is made.

## 5 - Inputs

Snow Load
ASCE 7-10
(Ref. 2-1)
Building Geometry
Field Measurements
(Ref. 2-4)

## 6 - Methodology

Roof Structure is checked for applicable Dead , Live , Snow loads.
7 - Limitations or restrictions on calculation applicability
This calculation is only applicable for residential building located at 4459 MacArthur Blvd NW, Washington, DC 20007

## Snow load :

Caclulate Flat Roof Snow Load :

Ground Snow Load from for District of Columbia region : $\mathrm{p}_{\mathrm{g}}:=30 \mathrm{psf}$
(Fig. 7-1, Ref 2-1)
Terrain category for open terrain with scattered obstructions having heights less than 30 ft :
Exposure factor for partially exposed roof :

$$
\mathrm{C}_{\mathrm{e}}:=1.0
$$

(Table 7-2, Ref 2-1)
Thermal factor for unheated structures :
$\mathrm{C}_{\mathrm{t}}:=1.1$
(Table 7-3, Ref 2-1)
Risk category of building : Risk_Cat:="II"
(Table 1.5-1, Ref 2-1)
Snow Importance factor : $\quad I_{\mathrm{S}}:=$ if Risk_Cat $=$ "I"


else
$\| 1.2 \quad \mathrm{I}_{\mathrm{s}}=1 \quad$ (Table 1.5-2, Ref. 2-1)
Flat roof snow load: $\mathrm{p}_{\text {flat }}:=\max \left(20 p s f \cdot \mathrm{I}_{\mathrm{s}}, 0.7 \cdot \mathrm{C}_{\mathrm{e}} \cdot \mathrm{C}_{\mathrm{t}} \cdot \mathrm{I}_{\mathrm{s}} \cdot \mathrm{p}_{\mathrm{g}}\right)=23.1 \mathrm{psf}($ Eq. 7.3-1, Ref. 2-1)
Minimum flat roof snow load :

$$
\begin{align*}
\mathrm{p}_{\min }:= & \text { if } \mathrm{p}_{\mathrm{g}} \leq 20  \tag{7.3.4,Ref.2-1}\\
& \| s f=20 p s f \\
& \| \mathrm{I}_{\mathrm{s}} \cdot \mathrm{p}_{\mathrm{g}} \\
& \text { else } \\
\| \mathrm{I}_{\mathrm{s}} \cdot(20 & p s f)
\end{align*}
$$

Flat roof snow load after minimum snow load is considered:

$$
\mathrm{p}_{\mathrm{f}}:=\max \left(\mathrm{p}_{\min }, \mathrm{p}_{\mathrm{flat}}\right)=23.1 p s f
$$

Roof slope factor :

$$
\mathrm{C}_{\mathrm{s}}:=1.0
$$

(Fig. 7-2b, Ref. 2-1)
Sloped roof :

$$
\begin{equation*}
\mathrm{p}_{\mathrm{s}}:=\mathrm{C}_{\mathrm{s}} \cdot \mathrm{p}_{\mathrm{f}}=23.1 \text { psf } \tag{Eq.7.4-1,Ref.2-1}
\end{equation*}
$$

Snow density : $\quad \gamma_{\text {snow }}:=\min \left(\left(0.13 \cdot \frac{\mathrm{p}_{\mathrm{g}}}{f t}+14 \cdot p c f\right), 30 p c f\right)=17.9 p c f \quad$ (Eq. 7.7-1, Ref. 2-1)

Height of balanced snow load (Snow drift height): $\mathrm{h}_{\mathrm{b}}:=\frac{\mathrm{p}_{\mathrm{s}}}{\gamma_{\text {snow }}}=1.291 \mathrm{ft}$
(7.7.1, Ref. 2-1)


Figure 6 - Configuration of snow drift (Fig. 7-8, Ref. 2-1)
Calculate Snow Drifts near parapet:
Parapet Height per architecture: $\quad \mathrm{h}_{\mathrm{pr}}:=42$ in
Clear height : $\quad h_{c}:=h_{p r}-h_{b}=2.209 f t$
Calculate_drift:= if $\frac{\mathrm{h}_{\mathrm{c}}}{\mathrm{h}_{\mathrm{b}}}<0.2$

$$
\begin{align*}
& \begin{array}{l}
\text { \|"No Drift" } \\
\text { else } \\
\text { elses" } \\
\\
\| \text { "Yes" }
\end{array} \quad \begin{array}{l} 
\\
\text { Calculate_drift }=\text { "Yes" }
\end{array}
\end{align*}
$$

Length of the roof upwind of the $\mathrm{l}_{\mathrm{u}}:=40 \mathrm{ft}$
drift :
If less than 25 ft use $\mathrm{lu}=25 \mathrm{ft} \quad \mathrm{l}_{\mathrm{u}}:=\max \left(\mathrm{l}_{\mathrm{u}}, 25 \mathrm{ft}\right)=40 \mathrm{ft}$
Ground Snow load:

$$
\begin{equation*}
\mathrm{p}_{\mathrm{g}}=30 \mathrm{psf} \tag{Fig.7-9,Ref.2-1}
\end{equation*}
$$

Height of snow drift : $\mathrm{h}_{\mathrm{d}}:=\frac{3}{4}\left(0.43 \cdot \sqrt[3]{\frac{1_{\mathrm{u}}}{f t}} \cdot \sqrt[4]{\frac{\mathrm{p}_{\mathrm{g}}}{p s f}+10}-1.5\right) f t=1.649 \mathrm{ft}$

$\mathrm{p}_{\mathrm{d}}:=$ if $\mathrm{h}_{\mathrm{d}} \leq \mathrm{h}_{\mathrm{c}}=1.649 \mathrm{ft}$


IBC section 1608 requires roofs with a slope less than $1 / 2^{\prime \prime}$ per 12 " to be designed for a rain-on-snow surcharge load as determined in accordance with ASCE 7, section 7.10. Section 7.10 does not require a rain-on-snow surcharge for locations where the ground snow load exceeds 20 psf; therefore, rain-on-snow surcharge is not required.

Summary:
Snow Load in flat parts: $\quad \mathrm{SL}_{\text {unif }}:=\max \left(\mathrm{p}_{\mathrm{g}}, \mathrm{p}_{\mathrm{f}}\right)=30 p s f$
Snow drift: $\quad \mathrm{SD}:=\mathrm{p}_{\mathrm{d}} \cdot \gamma_{\text {snow }}=29.512$ psf $\quad$ Max Drift lendth : $\mathrm{w}=6.595 \mathrm{ft}$


Figure 7 - Snow drift equivalent load
Equivalent Snow Drift: $\quad \mathrm{SD}_{\text {eqv }}=97.316 \mathrm{plf}$
Distance of equivalent load to parapet : $\mathrm{SD}_{\text {dist }}=2.198 \mathrm{ft}$


## Project Information

Project Title : Roof structure of existing building
Description :
I.D. :

Address : 4459 MacArthur Blvd NW, Washington, DC 20007
Project Leader : Y. Aliabadi
Phone : 240-678-5399 Fax : eMail: aastructure@gmail.com
Project Notes

Project Title: Roof structure of existing building
Engineer: Y. Aliabadi
Project ID:
Tel : 240-678-5399
Project Descr:
Email : aastructure@gmail.com

## Wood Beam

File: Roof Check.ec6

DESCRIPTION: Roof Joists check for Bending

## CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16
Load Combination Set : IBC 2018
Material Properties

| Analysis Method : Allowable Stress Design | Fb + | 1300 psi | E Modulus of Elasticity |  |
| :--- | :--- | :--- | :--- | :--- |
| Load Combination IBC 2018 | $\mathrm{Fb}-$ | 1300 psi | Ebend- xx | 1700 ksi |
|  |  | Fc-Prll | 1700 psi | Eminbend -xx |



## Applied Loads

Service loads entered. Load Factors will be applied for calculations.
Load for Span Number 1
Uniform Load: $\mathrm{D}=0.0070, \mathrm{~S}=0.0270 \mathrm{ksf}$, Tributary Width $=2.0 \mathrm{ft}$, (Snow Load)
Load for Span Number 2

$$
\text { Uniform Load : D }=0.0070, \mathrm{~S}=0.0270 \mathrm{ksf} \text {, Tributary Width }=2.0 \mathrm{ft} \text {, (Snow Load) }
$$

| DESIGN SUMMARY |  |  |  | Design OK |
| :---: | :---: | :---: | :---: | :---: |
| Maximum Bending Stress Ratio Section used for this span | 0.999. 1 | mum Shear Stress Ratio | $=$ | 0.404 : 1 |
|  | $2.0 \times 6.0$ | Section used for this span |  | 2.0 6.0 |
|  | 1,493.87psi |  | = | 67.36 psi |
|  | 1,495.00psi |  | $=$ | 166.75 psi |
| Load Combination | +D+S | Load Combination |  | +D+S |
| Location of maximum on span | 13.500ft | Location of maximum on span | = | 13.047 ft |
| Span \# where maximum occurs | Span \# 1 | Span \# where maximum occurs | = | Span \# 1 |
| Maximum Deflection |  |  |  |  |
| Max Downward Transient Deflection | 0.290 in Ratio = | $557>=240$ |  |  |
| Max Upward Transient Deflection | 0.000 in Ratio $=$ | $0<240$ |  |  |
| Max Downward Total Deflection | 0.366 in Ratio $=$ | $442>=180$ |  |  |
| Max Upward Total Deflection | -0.001 in Ratio $=$ | 77873 >=180 |  |  |

Maximum Forces \& Stresses for Load Combinations

| Load Combination | Max Stress Ratios |  |  | $C_{d}$ | $\mathrm{C}_{\text {FN }}$ | Ci | $C_{r}$ | $C_{m}$ | $\mathrm{C}_{\text {t }}$ | $C_{L}$ | Moment Values |  |  | Shear Values |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment Length | Span \# | M | V |  |  |  |  |  |  |  | M | fb | F'b | V | fv | F'v |
| D Only |  |  |  |  |  |  |  |  |  |  |  |  | 0.00 | 0.00 | 0.00 | 0.00 |
| Length $=13.50 \mathrm{ft}$ | 1 | 0.263 | 0.106 | 0.90 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.31 | 307.56 | 1170.00 | 0.11 | 13.87 | 130.50 |
| Length $=13.0 \mathrm{ft}$ | 2 | 0.263 | 0.106 | 0.90 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.31 | 307.56 | 1170.00 | 0.11 | 13.87 | 130.50 |
| +D+S |  |  |  |  | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 0.00 | 0.00 | 0.00 | 0.00 |
| Length $=13.50 \mathrm{ft}$ | 1 | 0.999 | 0.404 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.49 | 1,493.87 | 1495.00 | 0.54 | 67.36 | 166.75 |
| Length $=13.0 \mathrm{ft}$ | 2 | 0.999 | 0.404 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.49 | 1,493.87 | 1495.00 | 0.53 | 67.36 | 166.75 |
| +D+0.750S |  |  |  |  | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 0.00 | 0.00 | 0.00 | 0.00 |
| Length $=13.50 \mathrm{ft}$ | 1 | 0.801 | 0.324 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.20 | 1,197.30 | 1495.00 | 0.43 | 53.99 | 166.75 |
| Length $=13.0 \mathrm{ft}$ | 2 | 0.801 | 0.324 | 1.15 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.20 | 1,197.30 | 1495.00 | 0.42 | 53.99 | 166.75 |
| +0.60D |  |  |  |  | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |  |  | 0.00 | 0.00 | 0.00 | 0.00 |
| Length $=13.50 \mathrm{ft}$ | 1 | 0.089 | 0.036 | 1.60 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.18 | 184.54 | 2080.00 | 0.07 | 8.32 | 232.00 |

Tel : 240-678-5399
Project Title: Roof structure of existing building Engineer: Y. Aliabadi
Project ID:
Project Descr:
Email : aastructure@gmail.com

## Wood Beam

DESCRIPTION: Roof Joists check for Bending

| Load Combination | Max Stress Ratios |  |  | $\mathrm{C}_{\text {d }}$ | $\mathrm{C}_{\text {F/V }}$ | $\mathrm{C}_{\mathrm{i}}$ | $\mathrm{C}_{r}$ | $\mathrm{C}_{\mathrm{m}}$ | $\mathrm{C}_{\text {t }}$ | $C_{L}$ | Moment Values |  |  | Shear Values |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Segment Length | Span \# | M | V |  |  |  |  |  |  |  | M | fb | F'b | V | fv | F'v |
| Length $=13.0 \mathrm{ft}$ | 2 | 0.089 | 0.036 | 1.60 | 1.000 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 0.18 | 184.54 | 2080.00 | 0.07 | 8.32 | 232.00 |

## Overall Maximum Deflections

| Load Combination | Span | Max. "-- Defl | Location in Span | Load Combination | Max. "+" Defl | Location in Span |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +D+S | 1 | 0.3657 | 5.807 |  | 0.0000 | 0.000 |
| +D+S | 2 | 0.2810 | 7.626 | +D+S | -0.0009 | 0.218 |
| Vertical Reactions |  | Support notation : Far left is \#1 |  |  | Values in KIPS |  |
| Load Combination |  |  | 1 Support 2 | Support 3 |  |  |
| Overall MAXimum |  |  | 481.127 | 0.327 |  |  |
| Overall MINimum |  |  | 277 0.895 | 0.260 |  |  |
| D Only |  |  | 0.232 | 0.067 |  |  |
| +D+S |  |  | 1.127 | 0.327 |  |  |
| +D+0.750S |  |  | 279 0.903 | 0.262 |  |  |
| +0.60D |  |  | 0.139 | 0.040 |  |  |
| S Only |  |  | 277 0.895 | 0.260 |  |  |


[^0]:    ${ }^{1}$ Transcripts of ANC 3D's January and February 2021 meetings are available at www.anc3d.org. When completed, a transcript of the April 7, 2021 meeting will be available at www.anc3d.org.

[^1]:    ${ }^{2}$ See ANC 3D, Ltr. to Attorney General Racine and Acting Director Palacio regarding Alleged Fair Housing Violations, Feb. 3, 2021, https://resolutions.anc.dc.gov/AttachmentsFiles/13/OAG OHR\%20Letter\%20\%20Final TM 20210302052732 PM.pdf.
    ${ }^{3}$ ANC 3D strongly rejects the notion that questions about the impact of proposed project on the structural integrity of the neighboring structure should simply be left to the "permitting process"

[^2]:    ${ }^{4}$ Section F-5201 of the Zoning Regulations provides that an application for a special exception "shall not have a substantially adverse effect on the use or enjoyment of any abutting or adjacent dwelling or property, in particular: . . . the privacy of use and enjoyment of neighboring properties shall not be unduly compromised."

