18708

KJ & ASSOCIATES (703) 449-1600
CIVIL AND LAND DEVELOPMENT ENGINEERING FAX (703) 449-1601
10650 Main Street, Suite 302, Fairfax, VA 22030-3814 e-mail kjanda@cox net

December 13, 2013

Mr. Edwin Jacobsen
40862 Robin Circle
Leesburg, VA 20175

Dear Mr. Jacobsen;

Based on a review on the drainage patterns and topographic conditions proposed for Lot 960, Square 1397 in Washington D.C., we have provided means and methods to control the stormwater runoff during, and after construction of the proposed single family residence.

The total land disturbance for the project is 4,950 square feet. Construction drainage and Siltation and Erosion Control considerations will be handled by such standard D.C. methods as Earth Dikes along the eastern and southern limits of land disturbance; Super Silt Fence along the northern limits of disturbance; and double Silt Fence is used to control siltation along the western limits of disturbance on the lot. Inlet Protection is proposed to be installed upon the existing storm sewer inlet at the entrance to the lot.

Post-construction drainage from the site, both originating from the site, and runoff from off-site sources, will be controlled by way of sheet flow and will be collected by the existing storm sewer inlet at the lot entrance. This inlet was originally designed to adequately collect run-off from Lot 960 and it's upstream off-site land. There is no change in on-site or off-site drainage patterns.

As the proposed improvement on Lot 960 include construction of on-site stormwater management facility (infiltration trench) to convey the run-off generated by the proposed house on the lot, construction of the proposed single family residence will not have any adverse effect on the adjacent lots.

Sincerely,

Kayvan Jaboori, P E
Owner

RECEIVED
D.C. OFFICE OF ZONING
2013 DEC 16 PM 12: 07

BOARD OF ZONING ADJUSTMENT
District of Columbia
CASE NO. 18708
EXHIBIT NO. 24

PCS PROFESSIONAL CONSULTING SERVICES
GEOTECHNICAL•ENVIRONMENTAL•TOXICOLOGY•GEOLOGY•HYDROGEOLOGY

Partners:
Gerald C. Davit, P.E.
James T. Kirkland, Ph.D., C.P.G.
Barbara M. Davit, Ph.D., Chemist

December 18, 1993

Mr. Timothy J. Ward
5505 Seminary Road
Falls Church, Virginia 22041

Subject: Geotechnical Engineering Services,
Proposed Residence, 4509 Foxhall Crescents Drive, N.W.
Washington D.C. (PCS Project #931107)

Gentlemen:

We have performed two test borings on this property to explore subsurface soil and groundwater conditions.

Water was not encountered during drilling or observed at completion when the augers were removed. About 24 hours after drilling observations were made in the open bore holes. No water was observed in boring # 1 to a depth of 20 feet. Water was observed in test boring # 2 at a depth of about 20 feet (EL 295±). Based on this data hydrostatic groundwater is believed to be about 12 feet below the proposed lower floor of the new residence at EL 307±.

Groundwater levels fluctuate due to many variables. However, since groundwater levels are presently below planed excavation levels, we would not anticipate construction of the proposed home to influence groundwater levels on this or adjacent sites. Surface water should be addressed in your site plans.

Additional details are include in our geotechnical report for this project. If you have any questions please contact the undersigned.

Bzd 15882
Rec'd. 12/22/93
Ex. No. 23

Very truly yours,
PROFESSIONAL CONSULTING SERVICES

Gerald C Davit

Gerald C. Davit

DISTRICT OF COLUMBIA
GERALD C. DAVIT
No 6944
REGISTERED
PROFESSIONAL ENGINEER

20626 Highland Hall Drive • Gaithersburg, Maryland 20879 • (301) 926-8569

Existing Conditions and Proposed Construction

The property is located at the end of Foxhall Crescents Drive. The proposed site is wooded and slopes up from about EL 300 at the existing street to EL 319 to the south. Slopes are typically less than 3H:1V. There is an abandon asphalt road along the south side of the proposed home site.

The proposed structure will consist of a three story single family home with lower level at about EL 307.67. The outside grade will slope from about EL 307 in the front to El 317 along the rear of the building.

Subsurface Exploration

Two test borings were performed in the abandon road on the high side of the proposed site at the approximate locations shown on Enclosure (1).

Results of the subsurface exploration are shown on the test boring logs in Enclosure (2). These logs indicate results of Standard Penetration Tests (SPT) performed in the borings by ASTM D-1586, visual descriptions per ASTM D-2488, estimated Uniform Soil Classification System (USCS) group symbols per ASTM D-2487 and laboratory natural moisture content (MC).

Water was not observed in the test borings during drilling or at completion. Water was observed in test boring No. 2 at a depth of about 20 feet the day after drilling was completed. Based on these observations, we anticipate hydrostatic groundwater levels were about EL 295, which is 12± feet below the proposed floor, at the time the borings were performed.

Water levels may fluctuate due to changes in precipitation, surface grading, seasons of the year and other causes. Perched water, trapped by clayey soils, may also develop periodically.

Soil Laboratory Testing

Results of laboratory classification tests on selected samples are included with the ASTM D-2487 descriptions in Enclosure (3). Some variation between visual and laboratory determined results is anticipated.

Soil Conditions

Soils disclosed by this exploration generally consist of residual materials that were formed by weathering of the parent bedrock. Typically these soils increase in density with depth and grade from soil to rock. Some colluvial soils may also be present near the ground surface and at lower elevations.

Boring No. 1 disclosed about 1.5 feet of fill that was probably part of the abandon road. Below this material the soils consist of firm silty and clayey sand and sandy silt. Refusal was not encountered.

Soils disclosed by Boring no. 2 are typical of the general weathering pattern in residual soils. This boring indicates firm to hard silty sand classified SM. Refusal on hard residual material was encountered at 24 feet. Remnants of the asphalt road were present at the ground surface.

Foundation Support

Based on available information, firm, natural silty sand is anticipated at and below the lower floor at EL 307.67. These materials are anticipated to be suitable for support of normal spread footings with a soil bearing pressure of at least 2500 psf.

Foundation subgrades should be observed by us after they are excavated and prior to placement of concrete to evaluate if the soils anticipated are encountered. If necessary modifications to the footing sizes, depth or other design aspects could be made during construction.

Foundation Walls

The walls of the lower level will extend up to 10 feet below exterior grade. These walls must be designed to resist lateral earth pressure. We recommend designing foundation walls below grade for an equivalent fluid pressure of 45 pcf. A perimeter exterior drain is recommended to prevent hydrostatic pressure on the walls. This is discussed later in this report.

We recommend backfill placed against the exterior walls generally consist of soils classified ML, SM, SP, SW, GM, GW or GP. In order to limit settlement of exterior grades that may cause water ponding and increased water seepage into subdrains we recommend the backfill be compacted to at least 90 % of the maximum dry density determined by the Standard Proctor, ASTM D-698 or AASHTO T-99. Areas under walks, or other soil supported structures should be compacted to at least 95 % of this standard.

The on site soils encountered above the floor grade are generally anticipated to be of suitable quality for reuse as backfill. However, the natural moisture content is high and we anticipate scarifying and drying of the on site soils would be required to obtain compaction. Since the site is relatively small for performing drying operations it may become more economical to import dryer material. Weather conditions at the time of construction will be a factor.

Floor Support

The lower level floor slab may be soil supported. We recommend a minimum of 4 inches of open graded aggregate about 3/8 to 3/4 inch size be placed below the floor slab. The aggregate base should be covered with a layer of polyethylene at least 6 mil thickness or equivalent.

Floor slab subgrades should be prepared by removing any soft or unsuitable material. The subgrades should be observed by us prior to placing the aggregate base.

Subdrains

The proposed construction is anticipated to be above the hydrostatic groundwater levels; however, the lower level walls and floor slab should be protected from potential surface water infiltration.

We recommend that both interior and exterior drains be provided. A sketch of the subdrainage system recommended is shown in Enclosure (4). We suggest design drawings be developed to show the details of the system including cleanout locations, drain discharge points and invert elevations. If requested we can prepare drawings or review and comment on drawings prepared by others.

Limitations

This report has been prepared based on information furnished at this time and our interpretation of data from a limited number and depth of test borings and laboratory tests. Additional exploration and testing may disclose differing results. More than one interpretation of available data is possible. Subsurface conditions may change between locations explored and soil and water conditions change with time.

Our services have been provided in accordance with generally accepted geotechnical engineering practice based on available information. No warranties or guarantees are made.

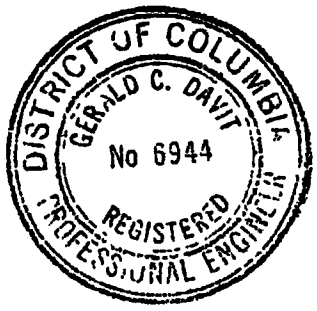
Samples obtained during this exploration will be retained for thirty days from the date of this report and then discarded unless other disposition is requested.

If additional services, design details, or clarification of any aspect of this report is desired please contact the undersigned.

Very truly yours,
PROFESSIONAL CONSULTING SERVICES

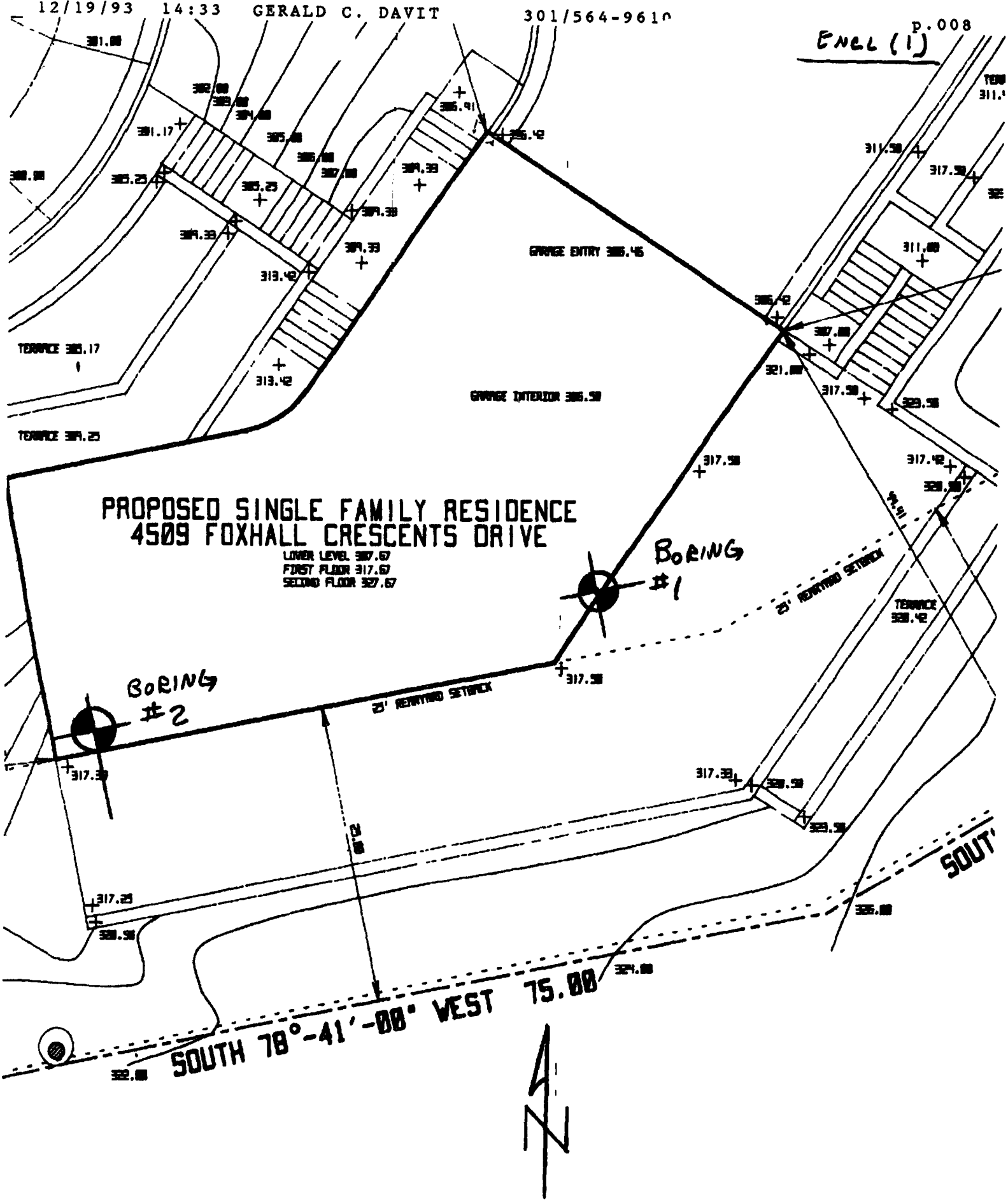
Gerald C Davit

Gerald C. Davit, P.E.



Enclosure:
1) Test Boring Locations (1 sheet)
2) Test Boring Logs (2 sheets)
3) Laboratory Testing (2 sheets)
4) Subdrain Sketch (1 sheet)

ENCL (1)



PROPOSED SITE PLAN

SCALE 1" = 10'-0"

TEST BORING LOG
PROFESSIONAL CONSULTING SERVICES

PROJECT
Ward Residence

Boring No. : 1
Ground Surface EL: 318±
PCS Project No. : 931107

Boring Contractor: Jamison Drilling Company
Drilling Method: 2¼" Hollow Stem Augers
Boring Location: As shown

Drilling Equipment: CME 45

Date Completed : 12-10-93
Sheet Number : 1 of 1

DEPTH (Ft)	SPT	VISUAL DESCRIPTION	USCS	STRA-TUM	MC (%)
0	2+5+5	FILL, contains; asphalt, crushed stone, clayey sand, wet			
1					
2		brown silty SAND, trace roots, contains mica, wet	SM		
3	4+4+8				
4					25
5		yellowish brown silty SAND, trace roots, contains mica, wet	SM		
6	6+5+6				
7					
8					
9	7+8+11				
10					38
11					
12					
13		reddish brown clayey SAND, contains mica, wet	SC		
14	5+7+11				
15					44
16					
17					
18		light brown & gray sandy SILT, contains mica, wet	ML		
19	6+11+12				
20					41
21					
22					
23					
24	6+7+8				
25		Boring terminated at 25.0'			

WATER OBSERVATIONS:

Encountered: None
Augers Pulled: None, Caved 20.0'
After 24 hours: None, Caved 20.0'

REMARKS:

TEST BORING LOG
PROFESSIONAL CONSULTING SERVICES

PROJECT
Ward Residence

Boring No. : 2
Ground Surface EL: 315±
PCS Project No. : 931107

Boring Contractor: Jamison Drilling Company
Drilling Method: 2¼" Hollow Stem Augers
Boring Location: As shown

Drilling Equipment: CME 45

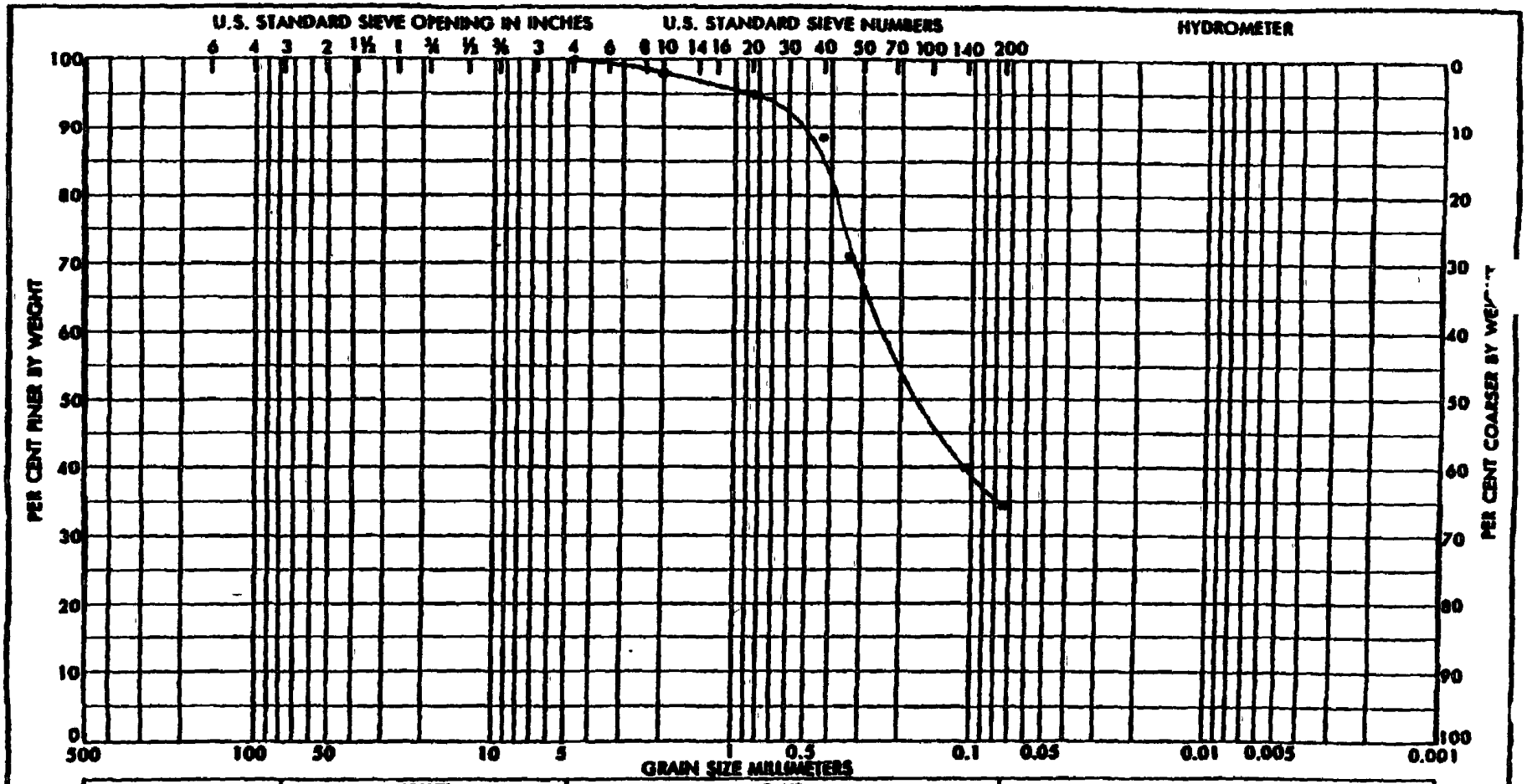
Date Completed : 12-10-93
Sheet Number : 1 of 1

DEPTH (Ft)	SPT	VISUAL DESCRIPTION	USCS	STRA-TUM	MC (%)
-	10+4+7	2"± Asphalt			
1		brown & gray silty SAND, with clayey sand layers,	SM		
-	--------	contains mica, wet			
2					
-	-------	brown & gray silty SAND, contains mica, moist to wet	SM		
3	5+6+8				
-					23
4	--------				
-					
5					
-	--------				
6	7+9+11				
-					
7	--------				
-					
8					
-	-------				
9	10+13+16				
-					13
10	-------				
-					
11					
-					
12					
-					
13					
-	-------				
14	16+24+29				
-					12
15	--------				
-					
16					
-					
17					
-					
18					
-	--------				
19	10+50/5½				
-					
20	--------				
-					
21					
-					
22					
-					
23					
-	--------				
24	50/5½"	Refusal at 24 feet			
-					
25					

WATER OBSERVATIONS:

Encountered: None
Augers Pulled: None, Caved 24.0'
After 24 hours: 20.0', Caved 24.0'

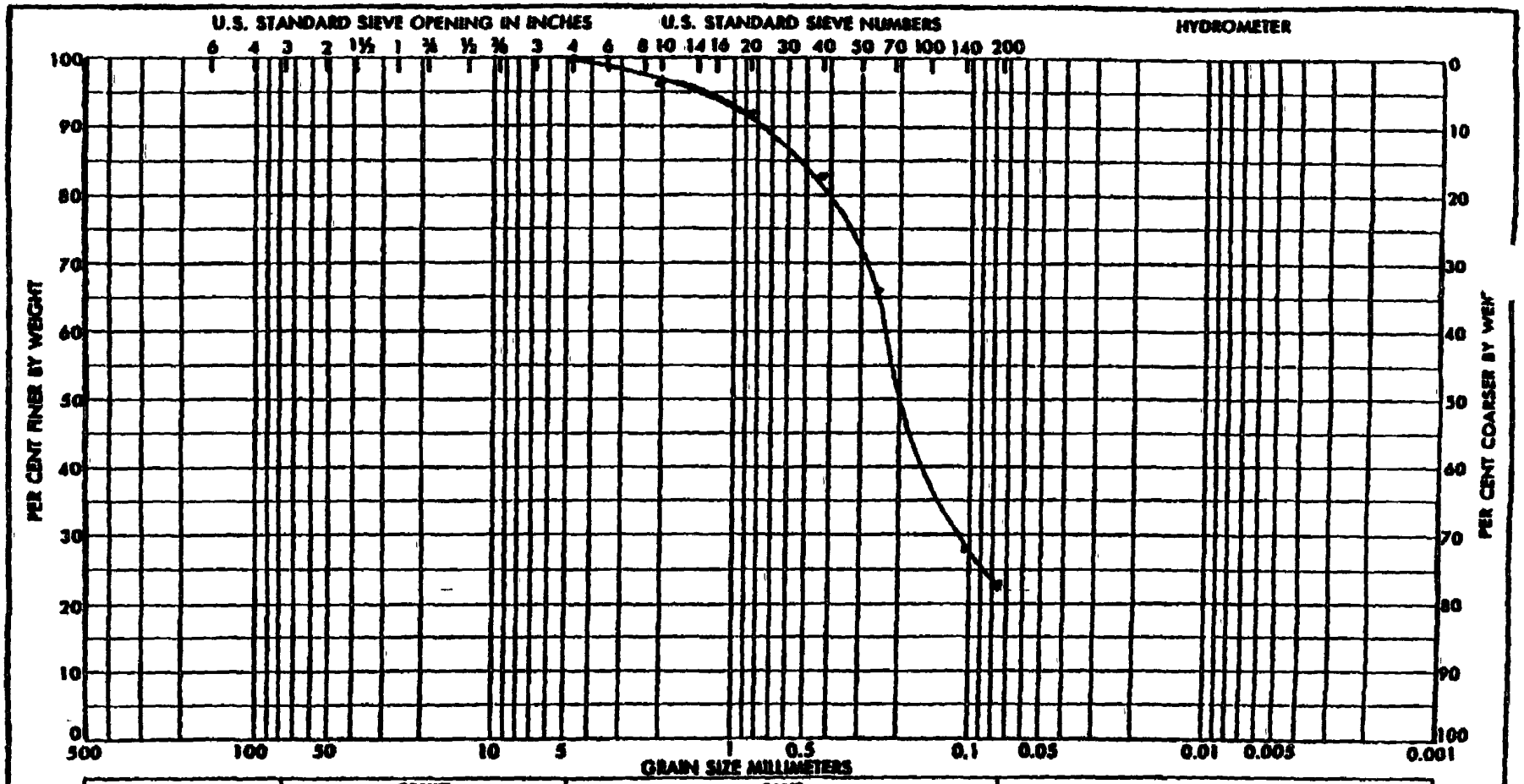
REMARKS:



SAMPLE NO	ELEV OR DEPTH	CLASSIFICATION	NAT W%	LL	PL	PI
		REDDISH BROWN,				
1	13.5-15.0'	CLAYEY SAND,		66	51	15
		CONTAINS MICA				

GRADATION CURVES

PROJECT WARD RESIDENCE
4509 FORNALL CRESCENTS DRIVE N.W.
AREA WASHINGTON D.C.
BORING NO. 1 @ 13.5-15.0'
DATE

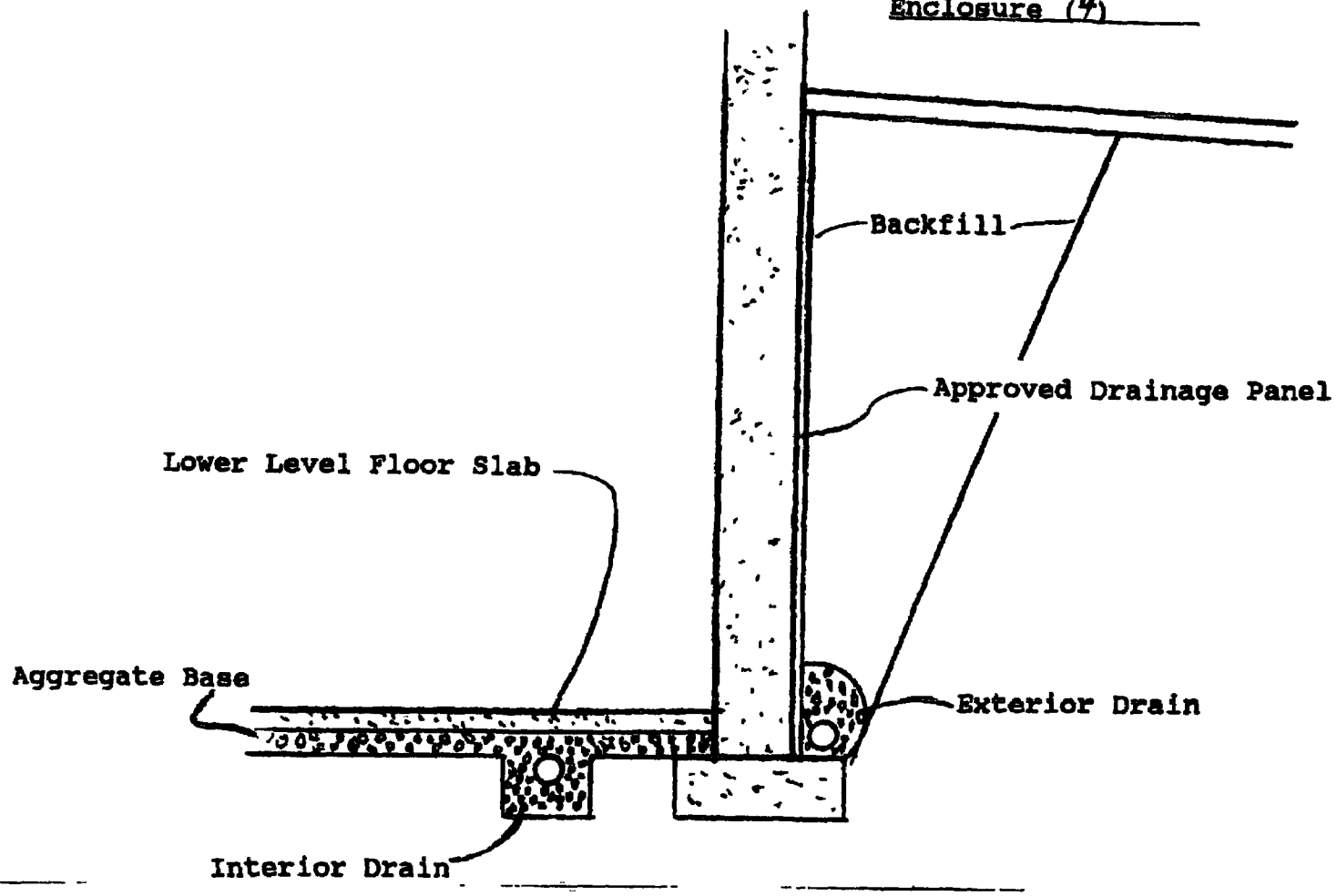


SAMPLE NO.	ELEV OR DEPTH	CLASSIFICATION	NAT W%	LL	PL	PI
		BROWN & GRAY SAND				
2	8.5'-10.0'	WITH SILT, CONTAINS		—	NP	NP
		MICA				

GRADATION CURVES

PROJECT WARD RESIDENCE
4509 FOXHALL CRESCENTS DRIVE N.W.
AREA WASHINGTON D.C.
BORING NO. 2 @ 8.5'-10.0'
DATE

Subdrainage Sketch
Enclosure (4)



General Notes:

1. Interior Drain 4" min. dia. corrugated PE pipe, ¼" slots, enveloped with 6" min. 3/8" open graded aggregate, drained by gravity or pump.

2. Exterior Drain 4" min. dia. corrugated PE pipe, ¼" slots, placed on footing against wall with invert 4" min. below bottom of inside slab, 6" min. 3/8" open graded aggregate above and side, aggregate separated from backfill by approved geotextile filter, drain by gravity.

3. Interior drains and/or exterior drains may be placed level or with positive slope to drain as shown on design drawings. Adjust grades during construction to prevent negative slopes.

4. Cap exterior grade over backfill with clayey soil and slope away from structures.

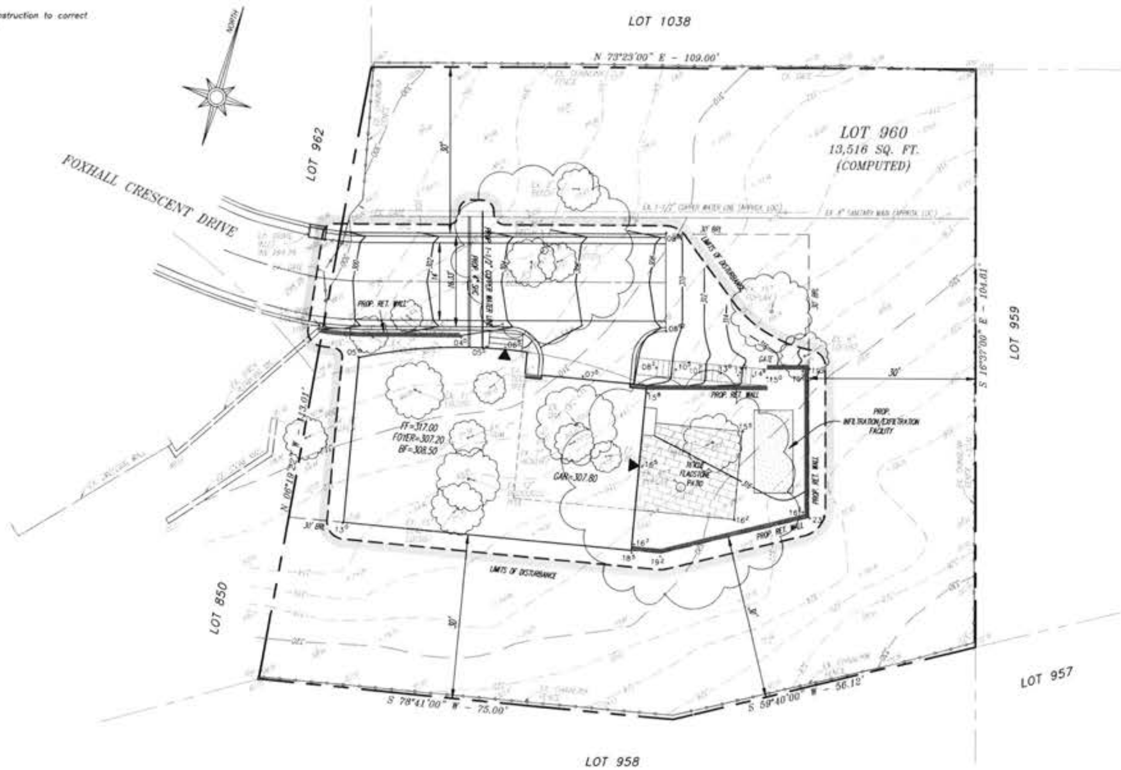
5. Provide cleanouts for drains as shown on design drawings.

6. Refer to geotechnical report for additional information.

NOTES:

- Owner: Amir Motlagh
  201 Berry Street, SE
  Vienna, VA 22180
- Property Address: 4509 Foxhall Crescent
  Washington, DC 20007
- Property Location: Lot #960, Square #1397
- Developer: Jacobsen Builders
  40862 Robinson Circle
  Leesburg, VA 20175
- All construction shall conform to DDOE and/or DDOT specifications.
- Only those utilities visible from the ground surface have been indicated on these plans. The location and depth of utilities shall be verified by contractor.
- This property is served by public water and sewer.
- No title report furnished.
- Recommendation for materials and methods of construction to correct existing soil conditions is not a part of this plan.
- F.E.M.A./F.I.R.M. DATA

  MAP ITEM ID.: 1100010012C
  FLOOD ZONE: X
  EFFECTIVE DATE: 09/27/2010



NO.	DESCRIPTION	A/R	APPROVED	DATE

REVISION APPROVED BY DIVISION OF DESIGN REVIEW

SURVEY	JARRETT SURVEYS, INC.
DESIGN	KJ
DRAWN	KJ
CHECKED	KJ

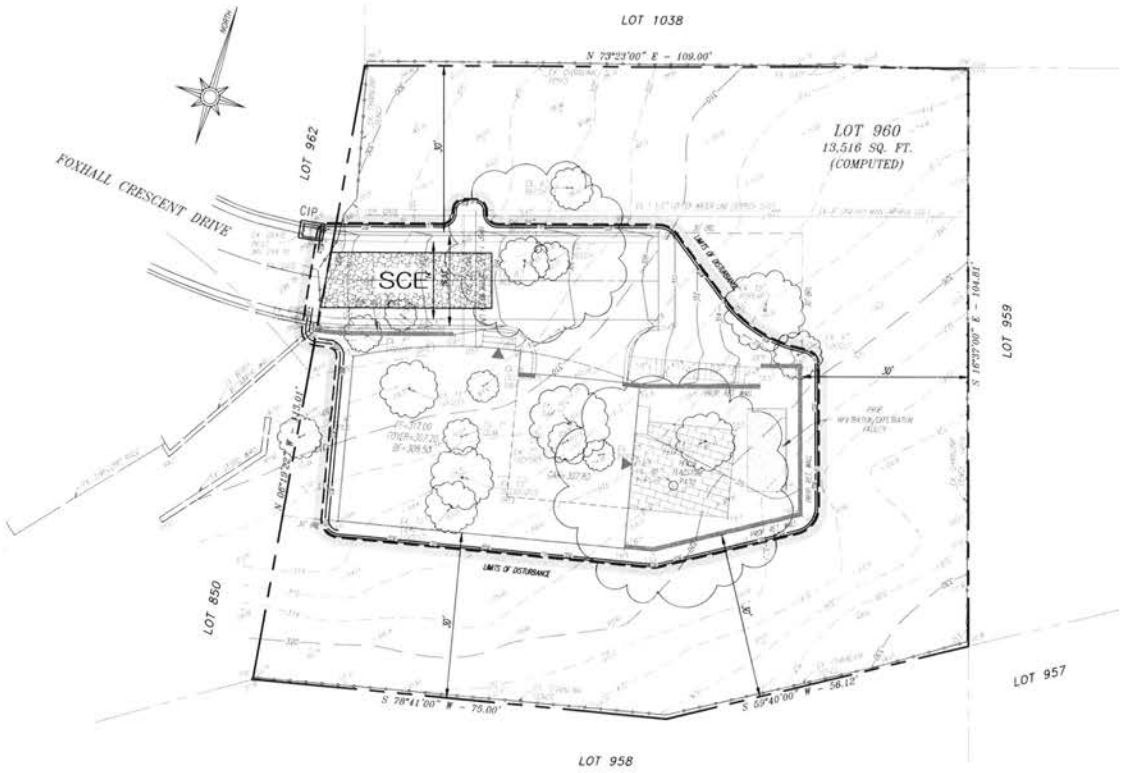
KJ & ASSOCIATES
CIVIL AND LAND DEVELOPMENT ENGINEERING
10650 MAIN STREET, SUITE 302 (703) 449-1600
FAIRFAX, VA 22030-3814 FAX (703) 449-1601
e-mail: kjanda@cox.net
www.kjandassociatesengineering.com

DATE	MAY 2013
SCALE	HORZ: 1"=10' VERT: N/A
SEAL	DISTRICT OF COLUMBIA · No.PE 905801 · CIVIL · PROFESSIONAL ENGINEER

SITE & GRADING PLAN

PROJECT: 4509 FOXHALL CRESCENTS DRIVE, N.W.
WASHINGTON, D.C. 20007
LOT 960, SQUARE 1397

JOB	2013-005
CADD	[illegible]
SHEET	1 OF 5



EROSION CONTROL NARRATIVE

THE CONSTRUCTION SHOWN HEREON PROPOSES THE CONSTRUCTION OF A HOUSE, RETAINING WALL AND ASSOCIATED FEATURES. THE EXISTING SITE CONDITION IS HEAVILY WOODED. IT IS THE INTENT OF THIS PLAN TO LIMIT THE AMOUNT OF LAND DISTURBANCE. EROSION AND SEDIMENT CONTROL DEVICES SHALL BE INSTALLED AS A FIRST ORDER OF CONSTRUCTION, IN ACCORDANCE WITH THE DETAIL AND LOCATION PROPOSED. EROSION AND SEDIMENT CONTROL DEVICES SHALL REMAIN IN PLACE AT ALL TIMES DURING CONSTRUCTION UNTIL FINAL STABILIZATION IS REACHED. PROVISIONS SHALL BE MADE TO MINIMIZE THE TRANSPORT OF SEDIMENT ONTO THE PAVED PUBLIC AND/OR PRIVATE ROADWAY BY CONSTRUCTION VEHICLES AND TO STABILIZE DISTURBED AREAS AS QUICKLY AS POSSIBLE. THE CONTROLS SHALL BE INSPECTED DAILY AND PARTICULARLY AFTER EACH RAINFALL EVENT. ANY NECESSARY REPAIRERS OR CLEANUP REQUIRED TO MAINTAIN EFFECTIVENESS OF THE CONTROLS SHALL BE MADE BY THE END OF EACH DAY. STRAWBALE SHALL BE PLACED ALONG THE WIDTH OF CONSTRUCTION ENTRANCE AT THE END OF EACH DAY. IF ADDITIONAL EROSION CONTROL DEVICES ARE DEEMED NECESSARY, THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSTALLATION. DISTURBED AREAS SHALL HAVE PERMANENT AND/OR TEMPORARY SOIL STABILIZATION APPLIED WITHIN SEVEN (7) DAYS AFTER FINAL GRADE IS REACHED. EROSION CONTROL SHALL BE REMOVED ONLY AT THE DIRECTION OF THE CITY INSPECTOR OR, AT LEAST, WITHIN THIRTY (30) DAYS OF FINAL SITE STABILIZATION. PERMANENT VEGETATION SHALL NOT BE CONSIDERED ESTABLISHED UNTIL A GROUND COVER IS REACHED THAT IS UNIFORMED AND MATURE ENOUGH TO SURVIVE AND THAT WILL INHIBIT EROSION. IF IT BECOMES NECESSARY, SEEDED AREAS SHALL BE FERTILIZED AND RESEEDED TO ENSURE A GOOD STAND OF GROUND COVER. TRAPPED SEDIMENT RESULTING FROM THE DEPOSITION OF THE CONTROLS SHALL BE DISPOSED OF PROPERLY TO PREVENT FURTHER SEDIMENTATION.

SEQUENCE OF CONSTRUCTION

1) CLEAR FOR AND INSTALL CONSTRUCTION ENTRANCE IN ACCORDANCE WITH THE EROSION AND SEDIMENT CONTROL PLAN (E&S).
2) INSTALL SUPER SILT FENCE (2 ROWS ALONG THE LOWER CLEARING LIMITS ON LOT) AS SHOWN ON THE E&S PLANS.
3) CLEAR AND GRUB WITHIN THE LIMITS OF DISTURBANCE ONLY. ALL DEBRIS TO BE TAKEN TO AN APPROVED DISPOSAL AREA.
4) LIMIT SOIL STOCKPILING ON-SITE TO EXTENT POSSIBLE.
5) BEGIN CONSTRUCTION ON HOUSE.
6) INSTALL WATER, SEWER, GAS AND CABLE CONNECTIONS AND EXTEND LINES TO FOUNDATIONS.
7) BACKFILL AND GRADE TO FINISHED ELEVATIONS AS SHOWN ON THE GRADING PLANS.
8) CONSTRUCT DRIVEWAY.
9) SEED AND MULCH ALL DISTURBED AREAS.
10) AFTER STABILIZATION, REMOVE SEDIMENT CONTROL MEASURES WITH THE APPROVAL OF THE CITY INSPECTOR.

GENERAL LAND CONSERVATION NOTES

1) NO DISTURBED AREA WHICH IS NOT ACTIVELY BEING WORKED UPON SHALL REMAIN DENUDED FOR MORE THAN 7 CALENDAR DAYS UNLESS OTHERWISE AUTHORIZED BY THE DIRECTOR.

2) ALL ESC CONTROL MEASURES SHALL BE PLACED PRIOR TO OR AS THE FIRST STEP IN GRADING.

3) ALL STORM AND SANITARY SEWER LINES NOT IN STREETS SHALL BE SEEDED AND MULCHED WITHIN 7 DAYS AFTER BACKFILL. NO MORE THAN 500' SHALL BE OPEN AT ANY ONE TIME.

4) ELECTRIC POWER, TELEPHONE AND GAS SUPPLY TRENCHES SHALL BE COMPACTED, SEEDED AND MULCHED WITHIN 7 DAYS.

5) ALL TEMPORARY EARTH BERMS, DIVERSIONS AND SEDIMENT CONTROL DAMS SHALL BE SEEDED AND MULCHED FOR TEMPORARY VEGETATIVE COVER IMMEDIATELY (AS SOON AS POSSIBLE BUT NOT LATER THAN 48 HOURS) AFTER COMPLETION OF GRADING. STRAW OR HAY MULCH IS REQUIRED. ALL SOIL STOCKPILES SHALL BE SEEDED AND MULCHED WITHIN 7 DAYS AFTER GRADING.

6) DURING CONSTRUCTION, ALL STORM SEWER INLETS SHALL BE PROTECTED BY SEDIMENT TRAPS, MAINTAINED AND MODIFIED DURING CONSTRUCTION PROGRESS AS REQUIRED.

7) ANY DISTURBED AREA NOT COVERED BY NOTE #1 ABOVE AND NOT PAVED, SODDED OR BUILT UPON BY NOVEMBER 1, OR DISTURBED AFTER THAT DATE, SHALL BE MULCHED IMMEDIATELY WITH HAY OR STRAW MULCH AT THE RATE OF 2 TONS/ACRE AND OVER-SEEDED BY APRIL 15.

8) AT THE COMPLETION OF ANY PROJECT CONSTRUCTION AND PRIOR TO BOND RELEASE, ALL TEMPORARY SEDIMENT CONTROLS SHALL BE REMOVED AND ALL DENUDED AREAS SHALL BE STABILIZED.

EROSION AND SEDIMENT CONTROL
STANDARDS AND SPECIFICATIONS FOR DUST CONTROL

1. The contractor shall conduct operations and maintain the project site so to minimize the creation and dispersion of dust. Dust control shall be used throughout the work at the site.

2. The contractor must provide clean water, free from salt, oil and other deleterious material to be used for on-site dust control.

3. The contractor shall supply water spraying equipment capable of accessing all work areas.

4. The contractor shall implement strict control measures during active construction periods on-site. These control measures will generally consist of water applications that shall be applied a minimum of once per day during dry weather or more often as required to prevent dust emission.

5. For water application to undisturbed soil surfaces, the contractor shall:

   A. Apply water with equipment consisting of tank, spray bar, pump with discharge pressure gauge;
   B. Arrange spray bar height, nozzle spacing and spray pattern to provide complete coverage of ground with water;
   C. Disperse water through nozzle on spray bar at 20 PSI (137.8 K Pa) minimum. Keep areas damp without creating nuisance conditions such as ponding.

6. For water application to soil surfaces during demolition and/or excavation, the contractor shall:

   A. Apply water with equipment consisting of a tank, pump with discharge gauge, hoses and mist nozzle;
   B. Locate tank and spraying equipment so that the entire excavation area can be misted without interfering with demolition and/or excavation equipment or operations. Keep areas damp without creating nuisance conditions such as ponding.
   C. Apply water spray in a manner to prevent movement of spray beyond the site boundaries.

DISTURBED AREA: 4,950 SQ. FT.
CUT: 560 CUBIC YARDS
FILL: 10 CUBIC YARDS

NO.	DESCRIPTION	ADDED	APPROVED	DATE
	REVISION APPROVED BY DIVISION OF DESIGN REVIEW			

SURVEY: JARRETT SURVEYS, INC.
DESIGN: K.J
DRAWN: K.J
CHECKED: K.J

KJ & ASSOCIATES
CIVIL AND LAND DEVELOPMENT ENGINEERING
10560 MAIN STREET, SUITE 302
FAIRFAX, VA 22030-3814
PHONE (703) 449-1500
FAX (703) 449-1501
e-mail: kjassociates@aol.net
www.kjassociatesengineering.com

DATE: DECEMBER 2013
SCALE: HORZ: 1"=10'
VERT: N/A
SEAL

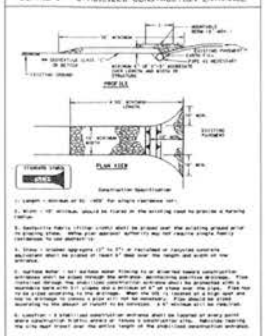
DISTRICT OF COLUMBIA
No.PE 900801
CIVIL
PROFESSIONAL ENGINEER

EROSION AND SEDIMENT CONTROL PLAN

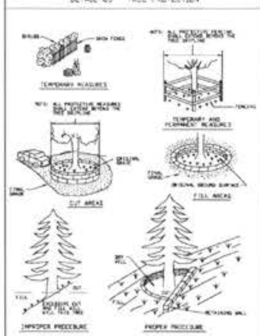
PROJECT: 4509 FOXHALL CRESCENTS DRIVE, N.W.
WASHINGTON, D.C. 20007
LOT 960, SQUARE 1397

JOB: 2013-005
CADD: D:\Projects\Foxhall Crescents\dwg\E&S.dwg
SHEET: 4 OF 5

DETAIL 1 - STABILIZED CONSTRUCTION ENTRANCE



DETAIL 03 - TREE PROTECTION



4.8 STANDARDS AND SPECIFICATIONS

FOR

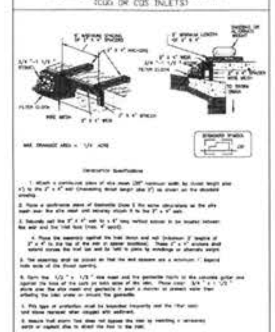
TREE PROTECTION

Definition

Purpose

Conditions Where Practice Applies

DETAIL 7C - CURB INLET PROTECTION (COG OR COS INLETS)



4.8 STANDARDS AND SPECIFICATIONS

FOR

SUPER SILT FENCE

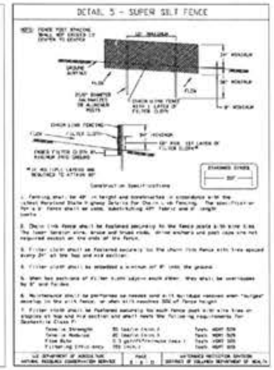
Definition

Purpose

Table 1 Design Criteria

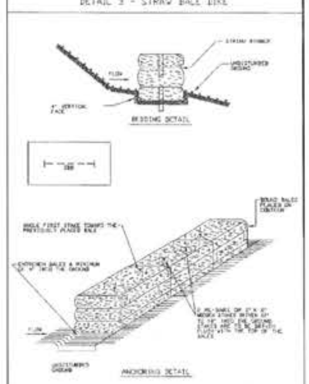
Construction Specifications

DETAIL 5 - SUPER SILT FENCE



SUPER SILT FENCE

DETAIL 3 - STRAW BALE DIKE



STRAW BALE DIKE

Construction Specifications

1. Bales shall be placed at the toe of a slope, on the contour, and in a row with the ends of each bale tightly abutting the adjacent bales.

2. Each bale shall be entrenched in the soil a minimum of 4" and placed so the bindings are horizontal.

3. Bales shall be securely anchored in place by either two 2" x 2" wooden stakes or re-bars driven through the bale 18" to 24" into the ground. The first stake in each bale shall be driven toward the previously laid bale at an angle to force the bales together. Stakes shall be driven flush with the top of the bale.

4. Straw bale dikes shall be inspected frequently and after each rain event and maintenance performed as necessary.

5. All bales shall be removed when the site has been stabilized. The trench where the bales were located shall be graded flush and stabilized.

NO.	DESCRIPTION	AB REVIEW	APPROVED	DATE

REVISION APPROVED BY DIVISION OF DESIGN REVIEW

SURVEY --
DESIGN KJ
DRAWN KJ
CHECKED KJ

KJ & ASSOCIATES
CIVIL AND LAND DEVELOPMENT ENGINEERING
10650 MAIN STREET, SUITE 302
FAIRFAX, VA 22030-3814
(703) 449-1600
FAX (703) 449-1601
e-mail: kjandassoc.net
www.kjandassociatesengineering.com

DATE DECEMBER 2013
SCALE HORZ: N/A VERT: N/A
SEAL DISTRICT OF COLUMBIA No.PE 905801 CIVIL PROFESSIONAL ENGINEER

EROSION AND SEDIMENT CONTROL DETAILS

PROJECT 4509 FOXHALL CRESCENTS DRIVE, N.W.
WASHINGTON, D.C. 20007
LOT 960, SQUARE 1397

JOB 2013-005
CADD D:\Projects\Foxhall Crescents\dwg\E&S.dwg
SHEET 5 OF 5

FOXHALL

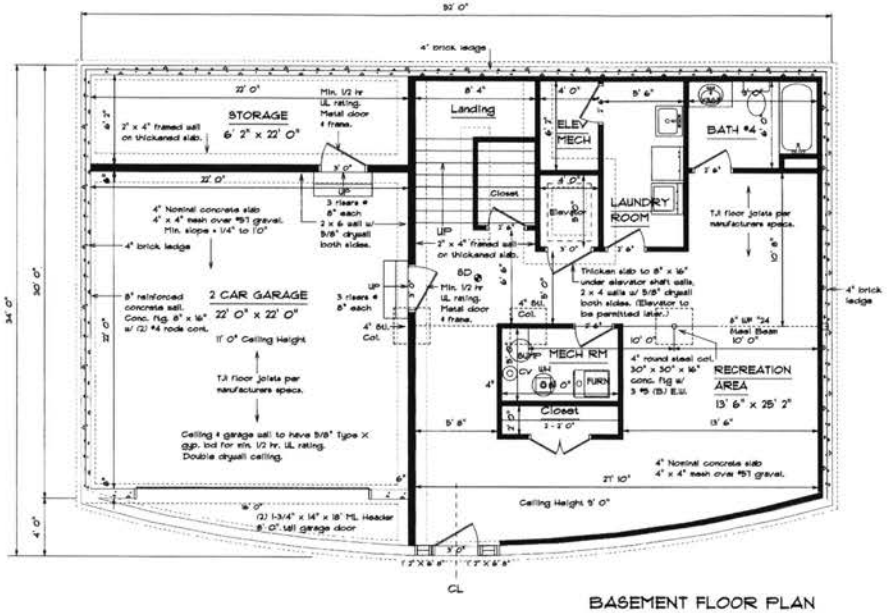
CS

4509 FOXHALL CRESCENTS DRIVE N.W.
WASHINGTON, DC 20007

AMIR MOTLAGH
4509 FOXHALL CRESCENTS DRIVE, NW
WASHINGTON, DC 20007
amirmotlagehotmail.com

December 5, 2013

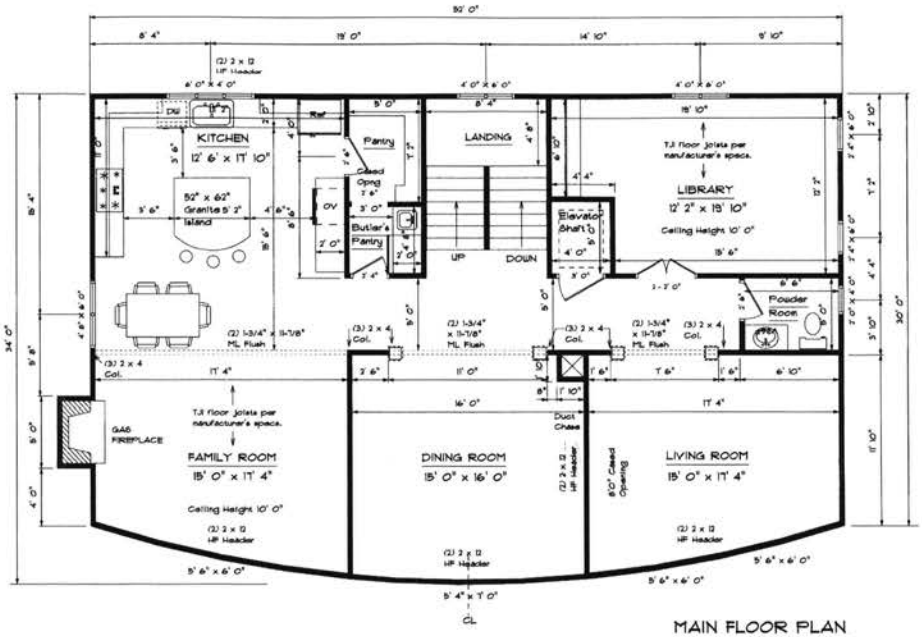
SHEET SCHEDULE



BASEMENT FLOOR PLAN

Scale 1/4" = 1' 0"

Headers (2) 2 x 10 unless otherwise specified

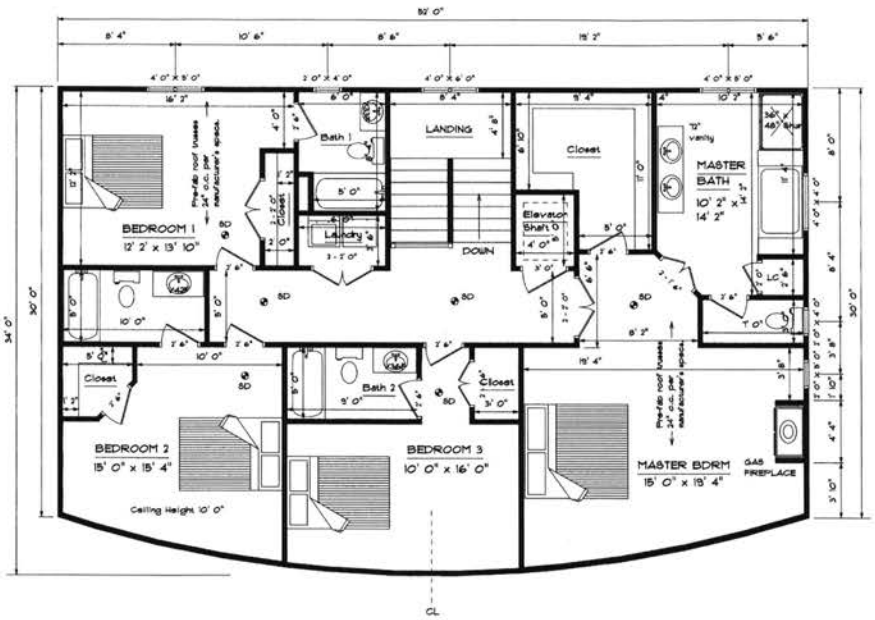


MAIN FLOOR PLAN

Scale 1/4" = 1' 0"

Headers (2) 2 x 10 unless otherwise specified

MODEL: Foxhall		
SCALE: 1/4" = 1'0", 1/8" = 1'0"	APPROVED BY:	DRAWN BY: CP
DATE: December 5, 2013		REVISED:
LOCATION: 4509 Foxhall Crescents Drive, NU Washington, DC 20007		
APPLICANT: Aeir Motlagh, Owner 4509 Foxhall Crescents Drive, NU Washington, DC 20007 aeirmotlagh@hotmail.com		DRAWING NUMBER: A.1



SECOND FLOOR PLAN

Scale 1/4" = 1' 0"

Headers (2) 2 x 10 unless otherwise specified

FRONT ELEVATION

Scale: 1/4" = 1'-0"

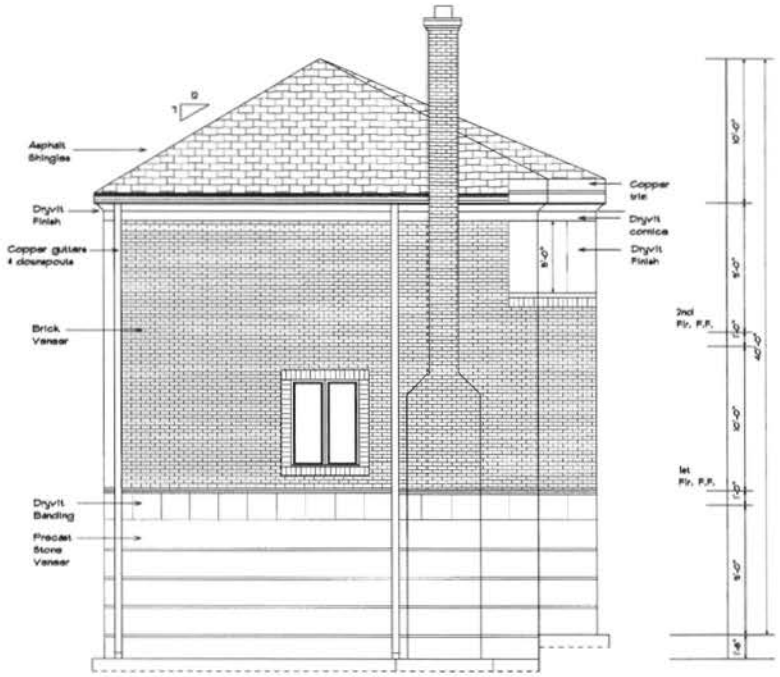
MODEL: Foxhall

SCALE: 1/4" = 1'0", 1/8" = 1'0"	APPROVED BY:	DRAWN BY: CP
DATE: December 5, 2013		REVISED:

LOCATION: 4509 Foxhall Crescents Drive, NW Washington, DC 20007

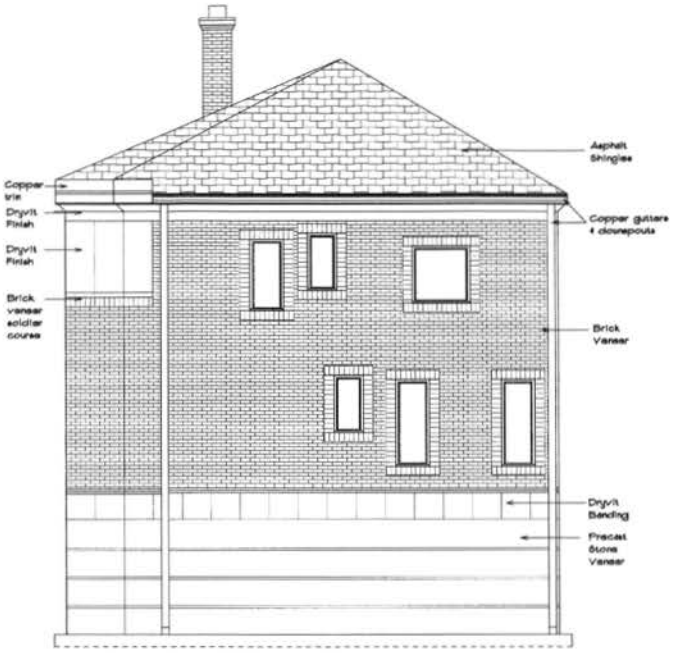
APPLICANT: Amir Motlagh, Owner
4509 Foxhall Crescents Drive, NW
Washington, DC 20007
amirmotlagh@gmail.com

DRAWING NUMBER: A.2



LEFT ELEVATION

SCALE: 1/4" = 1'-0"



RIGHT ELEVATION

SCALE: 1/4" = 1'-0"

MODEL: Foxhall

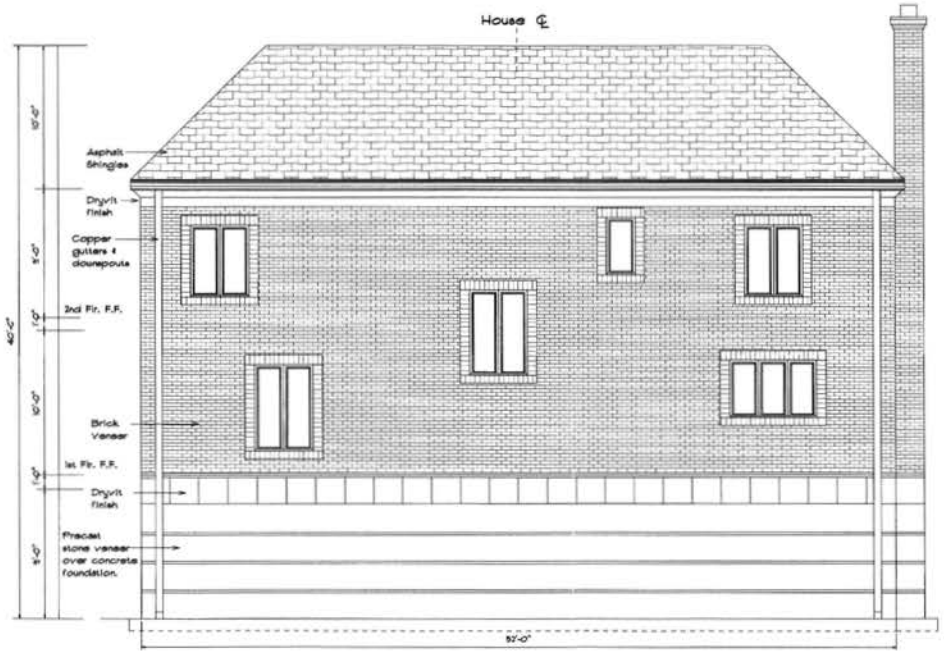
SCALE: 1/4" = 1'0", 1/8" = 1'0" | APPROVED BY: | DRAWN BY: CP

DATE: December 5, 2013 | REVISED:

LOCATION: 4805 Foxhall Crescents Drive, NU
Washington, DC 30007

APPLICANT: Amir Mostagh, Owner
4805 Foxhall Crescents Drive, NU
Washington, DC 30007
amirmostagh@yahoo.com

DRAWING NUMBER: A.3



REAR ELEVATION

Scale: 1/4" = 1'-0"

MODEL: Foxhall

SCALE: 1/4" = 1'-0", 1/8" = 1'-0" | APPROVED BY: | DRAWN BY: CP

DATE: December 9, 2013 | REVISED:

LOCATION: 4809 Foxhall Crescents Drive, NE
Washington, DC 20007

APPLICANT: Jack McGeigh, Owner
4809 Foxhall Crescents Drive, NE
Washington, DC 20007
[redacted]

DRAWING NUMBER: A.4